23020

Project Number

# PLANS FOR CONSTRUCTION

# DAVIS CREEK RESERVOIR

### ROAD IMPROVEMENT

#### **INDEX OF SHEETS**

SHEET NO. DESCRIPTION

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2-L1 TO 2-L8 PAVEMENT JOINTS AND SPOT ELEVATIONS

2-P PAINT PLAN 3 TO 6 PLAN AND PROFILE CULVERT CROSS SECTIONS 7 TO 9 X-1 TO X-60 CROSS SECTIONS

#### STANDARD PLANS

STANDARD DESCR PLAN NO.	.11 1101
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108-R5 SUPERELEVATION PLAN FOR CONCRETE & BITUMINOUS SURFACING

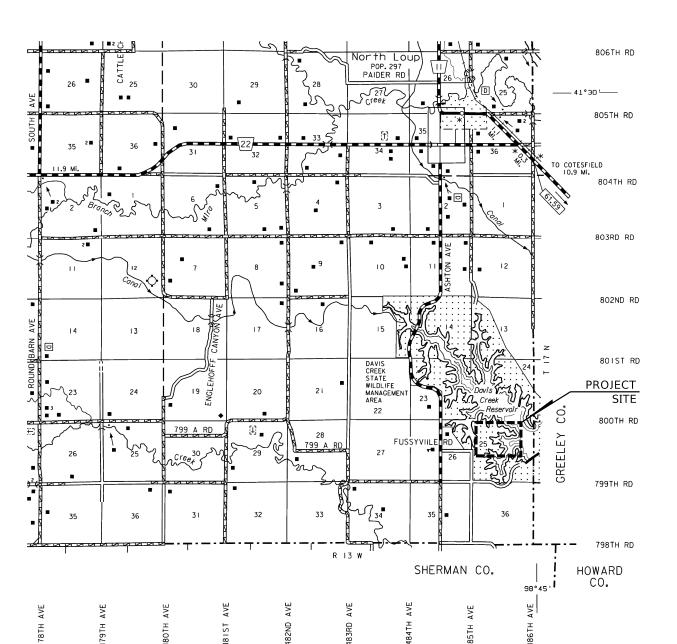
301-R12 (3 SHEETS) PAVEMENT DETAILS

329-R12 (4 SHEETS) 8 TO 16 INCH CONCRETE PAVEMENT (2 SHEETS) FLARED END SECTIONS FOR CULVERT PIPES 410-R4

(4 SHEETS) BEDDING AND BACKFILL REQUIREMENTS FOR CULVERT PIPE 411-R2

501-R7 (3 SHEETS) EROSION CONTROL 502-R2 (2 SHEETS) SILT FENCE DETAILS

(2 SHEETS) PAVEMENT MARKING 941-R1



THE 2017 EDITION OF THE NEBRASKA DEPT. OF TRANSPORTATION STANDARD SPECIFICATIONS AND THE PROJECT SPECIAL PROVISIONS APPLY TO THIS PROJECT.

HALF SIZE PLANS





REFERENCE POST NO. N/A TO REFERENCE POST NO. N/A

EXCEPTIONS: FROM STA. N/A TO STA. N/A

TOTAL NET LENGTH OF PROJECT: 5,106 FEET 0.967 MILES



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N

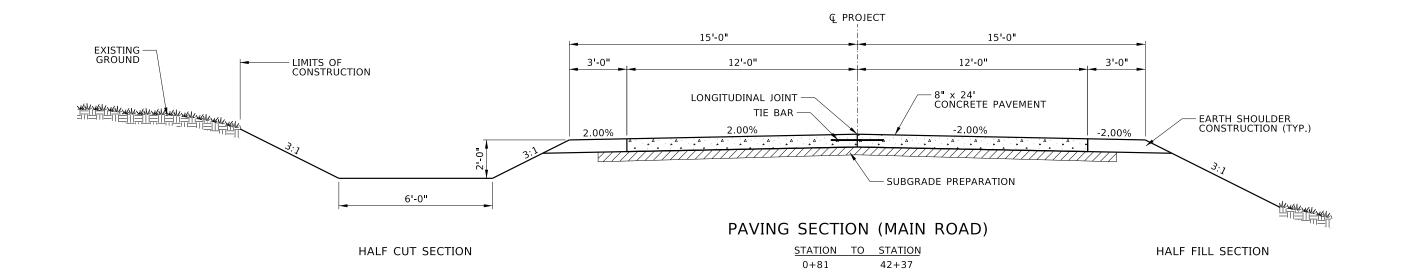
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**CROSS** 

3260 FOLKWAYS BLVD SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM

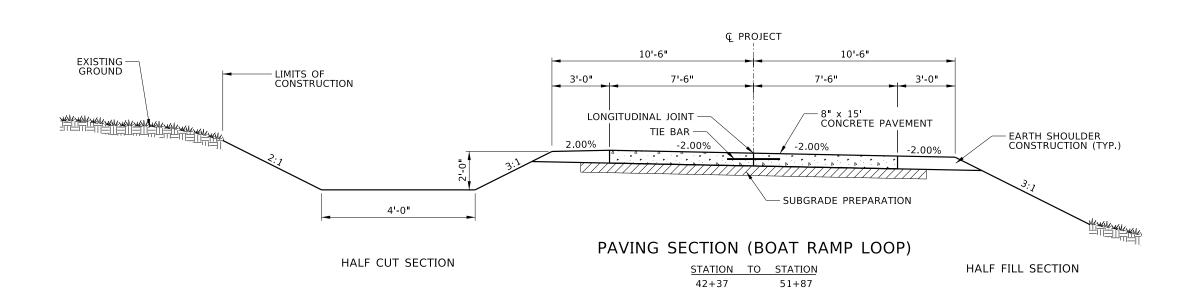
JOSHUA L.

## TYPICAL CROSS SECTION OF IMPROVEMENT



(UNLESS SHOWN OTHERWISE)

(UNLESS SHOWN OTHERWISE)



### LEGEND

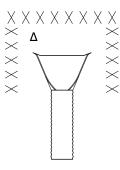
- — G — -	GAS LINE	0	MANHOLE
E	ELECTRICAL SERVICE	<u> </u>	MARSH
P	POWER LINE	百	OIL WELL
<u> —</u> ор —	OVERHEAD POWER LINE	$\langle \rangle$	PHOTO CODE POINT
— SAN —	SANITARY SEWER	P	POWER BOX
—— ss ——	STORM SEWER	<b></b>	POWER POLE
— т —	TELEPHONE LINE	P	POWER PULL BOX
—— TFO ——	FIBER OPTIC TELE. LINE		PROPANE TANK
— от —	OVERHEAD TELEPHONE LINE	R	R.O.W. MARKER
—— TV ——	CABLE TV LINE	•	ADVANCED R.R. WARNING SIGN
— отv —	OVERHEAD CABLE TV LINE	<b>}</b> •	RAILROAD WARNING
w	WATER LINE		RAILROAD TRACKS
<u> </u>	FENCE - CHAIN LINK		RETAINING WALL
×	FENCE - R.O.W. OR WIRE	$\epsilon$	SATELLITE DISH
—	FENCE - WOOD	0	SIGN
	FLOWLINE	*	TRAFFIC SIGNAL
_	CENTER LINE DRIVE	*	TRAFFIC SIGNAL/ST. LIGHT
B M	BENCH MARK	T	TELEPHONE BOX
•	CENTER PIVOT	FO	TELE. FIBER OPTICS BOX
$\odot$	CONTROL POINT	1	TELEPHONE PULL BOX
XXXXXXX	DIKE	•	TELEPHONE POLE
©	GAS METER	TV	TELEVISION BOX
$\bowtie$	GAS VALVE	ZWZ ZWZ	TREE - CONIFEROUS
-	GRID TICK	( )	TREE - DECIDUOUS
• • • • • •	GUARDRAIL	ã	TREE STUMP
۰	GUARD POST	<b>6</b> -	WATER (FIRE) HYDRANT
•	GUY POLE	H	WATER VALVE
$\longrightarrow$	GUY WIRE	<b>W</b>	WATER METER
;;;;; OR □;;;;;	LIGHT POLE	$\Leftrightarrow$	WELL
а	MAILBOX	Š	WINDMILL

#### **GENERAL NOTES:**

THE LOCATIONS OF ALL AERIAL AND UNDERGROUND UTILITY FACILITIES MAY NOT BE INDICATED IN THESE PLANS. UNDERGROUND UTILITIES, WHETHER INDICATED OR NOT WILL BE LOCATED AND FLAGGED BY THE UTILITIES AT THE REQUEST OF THE CONTRACTOR.

NO EXCAVATION WILL BE PERMITTED IN THE AREA OF UNDERGROUND UTILITIES UNTIL ALL SUCH FACILITIES HAVE BEEN LOCATED AND IDENTIFIED TO THE SATISFACTION OF ALL PARTIES. THE EXCAVATION MUST BE ACCOMPLISHED WITH EXTREME CARE IN ORDER TO AVOID ANY POSSIBILITY OF DAMAGE TO THE UTILITY FACILITY.

UPON COMPLETION OF GRADING OPERATIONS, SEEDING OF ALL DISTURBED AREAS SHALL BE PERFORMED BY THE CONTRACTOR, AS DIRECTED BY THE PROJECT MANAGER. SEE THE SPECIAL PROVISIONS FOR ADDITIONAL INFORMATION.



# COMPACTION REQUIREMENTS, CLASS III (SEE SPECIFICATIONS)

		DEPTH BELOW	DEPTH BELOW	PERCENT	MOISTURE REQUIREMENTS	
	SOIL TYPE	FINISHED GRADE	DENSITY	MIN.	MAX.	
EMBANKMENT/ROADWAY GRADING TO RECEIVE CONCRETE PAVEMENT	SILT-CLAY	UPPER 3 FEET	98 MIN.	OPT3%	OPT. +2%	
	SILT-CLAY	AT DEPTHS GREATER THAN 3 FEET	95 MIN.	OPT3%	OPT. +2%	
	GRANULAR	ALL DEPTHS	100 MIN.	**	**	
SUBGRADE PREPARATION, SHOULDER SUBGRADE PREPARATION (CONCRETE PAVEMENT)	SILT-CLAY	UPPER 6 INCHES OF SUBGRADE SOIL	98 MIN.	OPT3%	OPT. +2%	
	GRANULAR	UPPER 6 INCHES OF SUBGRADE SOIL	100 MIN.	**	**	
EMBANKMENT/ GRADING ON SECONDARY ROADS	ALL	ALL DEPTHS	CLASS II	SEE SPECIFICATIONS		
OTHER GRADING, INCLUDING DRIVES NOT SURFACED	ALL	ALL DEPTHS	CLASS I	SEE SPECIFICATIONS		

\*\* MOISTURE AS NECESSARY TO OBTAIN DENSITY
(CONTRACTOR SHALL ESTABLISH MOISTURE TARGET VALUE IN THE FIELD DURING COMPACTION PROCESS. ONCE ESTABLISHED,
ACCEPTABLE MOISTURE CONTENT SHALL BE +/- 2% OF TARGET VALUE.)

DESIGN EARTHWORK QUANTITIES*			(DOES NOT INCLUDE DRIVES AND DIKES)		
STATION	TO STATION	DESCRIPTION	EXCAVATION (CU. YDS.)	EMBANKMENT (CU. YDS.)	
0+81	23+00	PHASE 1	1,087	1,088	
23+00	33+00	PHASE 2	801	94	
33+00	51+87	PHASE 3	1,001	688	
		TOTAL	2,889	1,870	

NOTE: EARTHWORK NUMBERS SHOWN DO NOT INLUDE A BALANCE FACTOR.

BUILD SILT FENCE, STANDARD PLAN NO. 502					
STATION	SIDE	LENGTH (FT.)	TYPE		
1+33	Δ RT.	30	HIGH		
4+66	Δ RT.	30	HIGH		
15+05	Δ RT.	30	HIGH		
21+80	Δ LT.	30	HIGH		
31+50	Δ LT.	30	HIGH		
44+95	Δ RT.	30	HIGH		
47+17	Δ LT.	30	HIGH		
0+81 TO 2+50	* LT	169	LOW		
4+00 TO 6+00	* RT	200	LOW		
4+00 TO 10+00	* LT	600	LOW		
12+50 TO 13+50	* RT	100	LOW		
16+50 TO 18+00	* LT	150	LOW		
20+00 TO 23+00	* RT	300	LOW		
31+50 TO 32+00	* RT	50	LOW		
34+00 TO 35+00	* RT	100	LOW		
37+00 TO 40+00	* LT	300	LOW		
37+00 TO 40+00	* RT	300	LOW		
43+00 TO 45+00	* LT	200	LOW		
46+00 TO 47+00	* LT	100	LOW		

 $\Delta$  place at inlet of culvert as shown  $^{\ast}$  place along toe of fill slope

2-N

Project Number 23020

> INFORMATION 25-T17N-R13W ENERAL

RESERVOIR ROAD IMPROVEMENT

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### BASE BID

### PHASE 1

ITEM	QUANTITY	UNIT	ITEM	QUANTITY	UNIT
MOBILIZATION	1.000	LUMP SUM	MOBILIZATION	1.000	LUMP SUM
GENERAL CLEARING AND GRUBBING	1.000	LUMP SUM	GENERAL CLEARING AND GRUBBING	1.000	LUMP SUM
REMOVE CULVERT PIPE	83.000	LIN. FT.	EXCAVATION, ESTABLISHED QUANTITY	801.000	CU. YDS.
EARTHWORK MEASURED IN EMBANKMENT	1,088.000	CU. YDS.	12" CORRUGATED METAL PIPE	60.000	LIN. FT.
12" CORRUGATED METAL PIPE	64.000	LIN. FT.	12" FLARED-END SECTION	2.000	EACH
12" FLARED-END SECTION	4.000	EACH	8" CONCRETE PAVEMENT, CLASS 47B-3500	2,667.000	SQ. YDS.
8" CONCRETE PAVEMENT, CLASS 47B-3500	5,918.000	SQ. YDS.	SUBGRADE PREPARATION	2,667.000	SQ. YDS.
SUBGRADE PREPARATION	5,918.000	SQ. YDS.	8" CONCRETE PAVEMENT, CLASS 47B-3500		
8" CONCRETE PAVEMENT, CLASS 47B-3500			FOR INTERSECTIONS, DRIVES AND PARKING AREAS	578.000	SQ. YDS.
FOR INTERSECTIONS, DRIVES AND PARKING AREAS	1,551.000	SQ. YDS.	PREPARATION OF INTERSECTIONS, DRIVES		
PREPARATION OF INTERSECTIONS, DRIVES			AND PARKING AREAS	578.000	SQ. YDS.
AND PARKING AREAS	1,551.000	SQ. YDS.	EARTH SHOULDER CONSTRUCTION	20.000	STA.
EARTH SHOULDER CONSTRUCTION	44.380	STA.	SEEDING, TYPE "A"	0.500	ACRES
SEEDING, TYPE "A"	1.100	ACRES	COVERCROP SEEDING	0.500	ACRES
COVERCROP SEEDING	1.100	ACRES	TEMPORARY SILT FENCE	600.00	LIN. FT.
TEMPORARY SILT FENCE	800.000	LIN. FT.	FABRIC SILT FENCE, HIGH POROSITY	60.000	LIN. FT.
FABRIC SILT FENCE, HIGH POROSITY	120.000	LIN. FT.	FABRIC SILT FENCE, LOW POROSITY	50.000	LIN. FT.
FABRIC SILT FENCE, LOW POROSITY	1,519.000	LIN. FT.	REMOVABLE SPEED BUMP	1.000	EACH
REMOVABLE SPEED BUMP	1.000	EACH	PRE-CAST CONCRETE CURB STOP	16.000	EACH
PRE-CAST CONCRETE CURB STOP	10.000	EACH			

ALTERNATE 1

SUMMARY OF QUANTITIES

PHASE 2

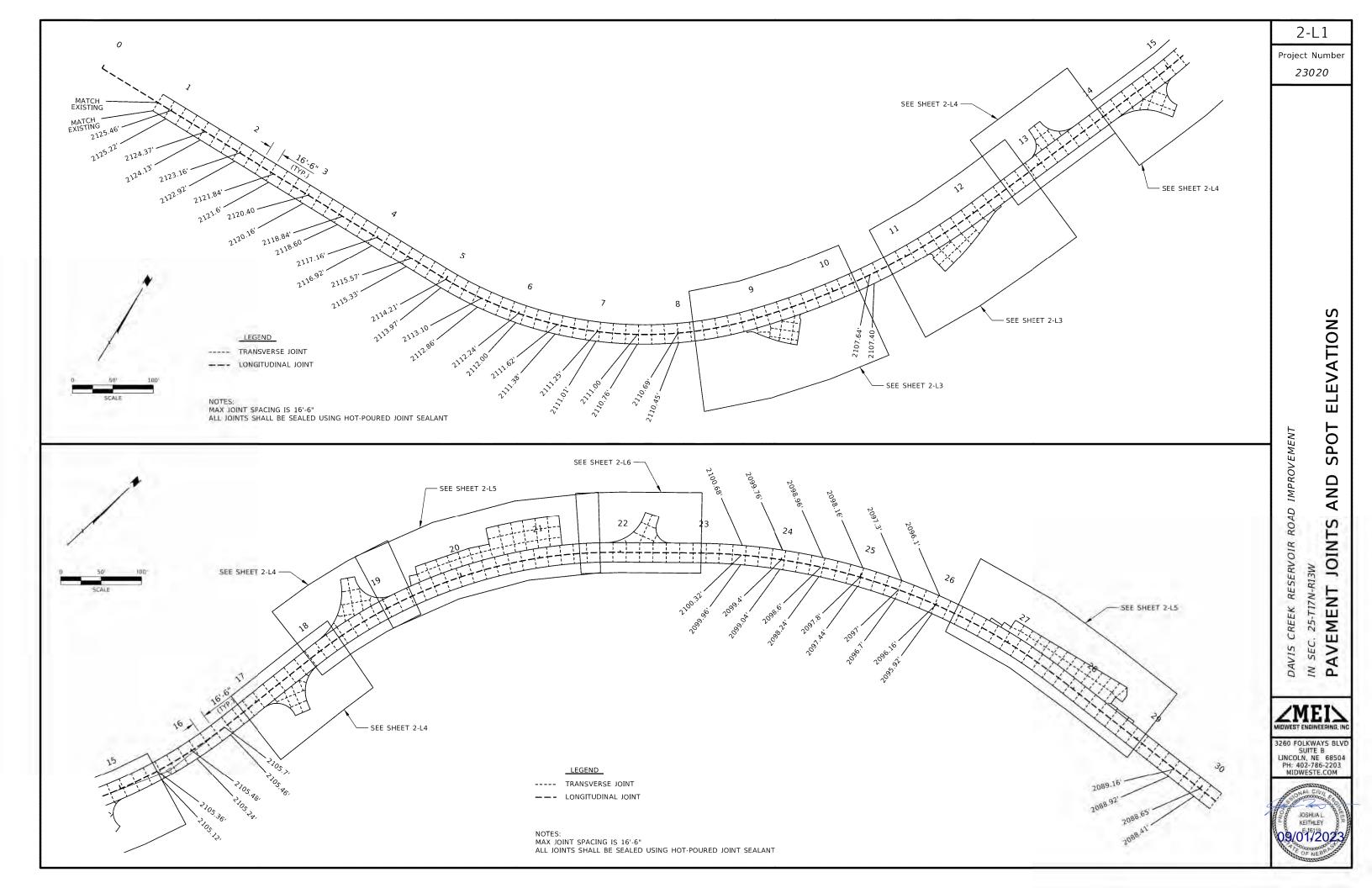
### ALTERNATE 2

## SUMMARY OF QUANTITIES

### PHASE 3

ITEM	QUANTITY	UNIT
MOBILIZATION	1.000	LUMP SUM
GENERAL CLEARING AND GRUBBING	1.000	LUMP SUM
EXCAVATION, ESTABLISHED QUANTITY	1,001.000	CU. YDS.
12" CORRUGATED METAL PIPE	24.000	LIN FT.
12" FLARED-END SECTION	2.000	EACH
8" CONCRETE PAVEMENT, CLASS 47B-3500	4,066.000	SQ. YDS.
SUBGRADE PREPARATION	4,066.000	SQ. YDS.
8" CONCRETE PAVEMENT, CLASS 47B-3500		
FOR INTERSECTIONS, DRIVES AND PARKING AREAS	1,884.000	SQ. YDS.
PREPARATION OF INTERSECTIONS, DRIVES		
AND PARKING AREAS	1,884.000	SQ. YDS.
EARTH SHOULDER CONSTRUCTION	38,360	STA.
SEEDING, TYPE "A"	0.900	ACRES
COVERCROP SEEDING	0.900	ACRES
TEMPORARY SILT FENCE	600.000	LIN. FT.
PERMANENT PAVEMENT MARKING, PAINT	630.000	LIN. FT.
FABRIC SILT FENCE, HIGH POROSITY	60.000	LIN. FT.
FABRIC SILT FENCE, LOW POROSITY	1,000.000	LIN. FT.
PRE-CAST CONCRETE CURB STOP	14.000	EACH

## SUMMARY OF QUANTITIES



Project Number 23020

> **ELEVATIONS** SPOT AND

IN SEC. 25-TIZN-RI3W
PAVEMENT JOINTS

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JOSHUA L. KEITHLEY

ELEVATIONS

2-L3

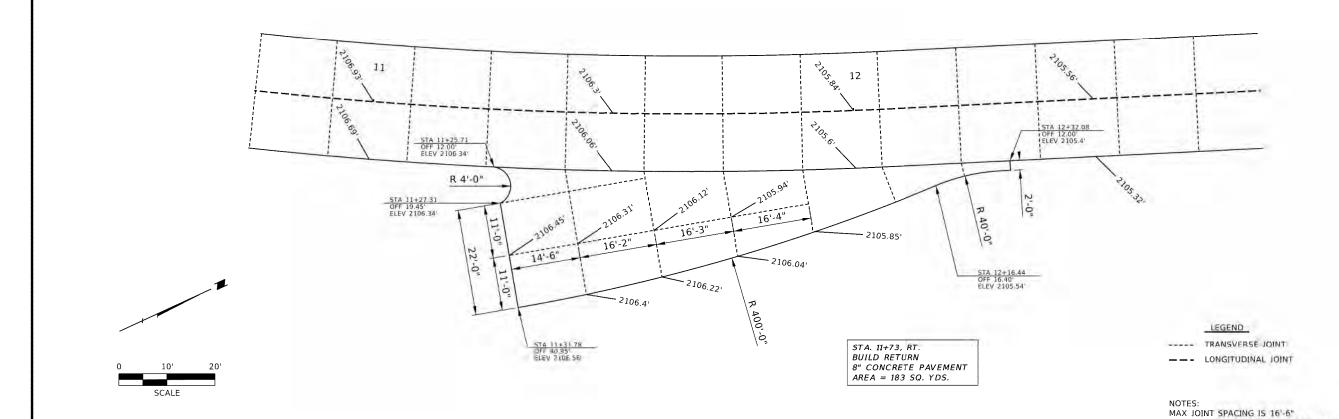
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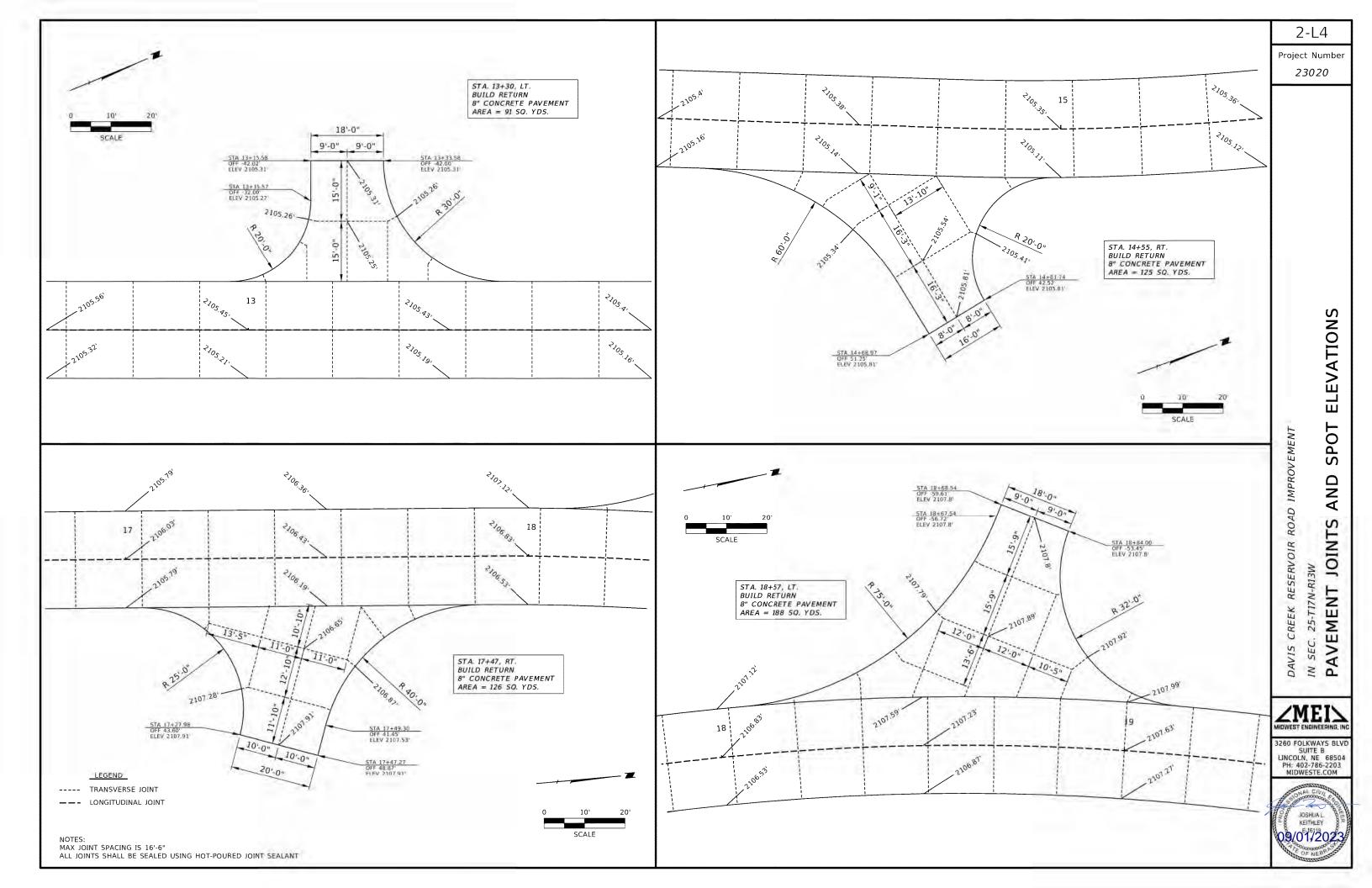
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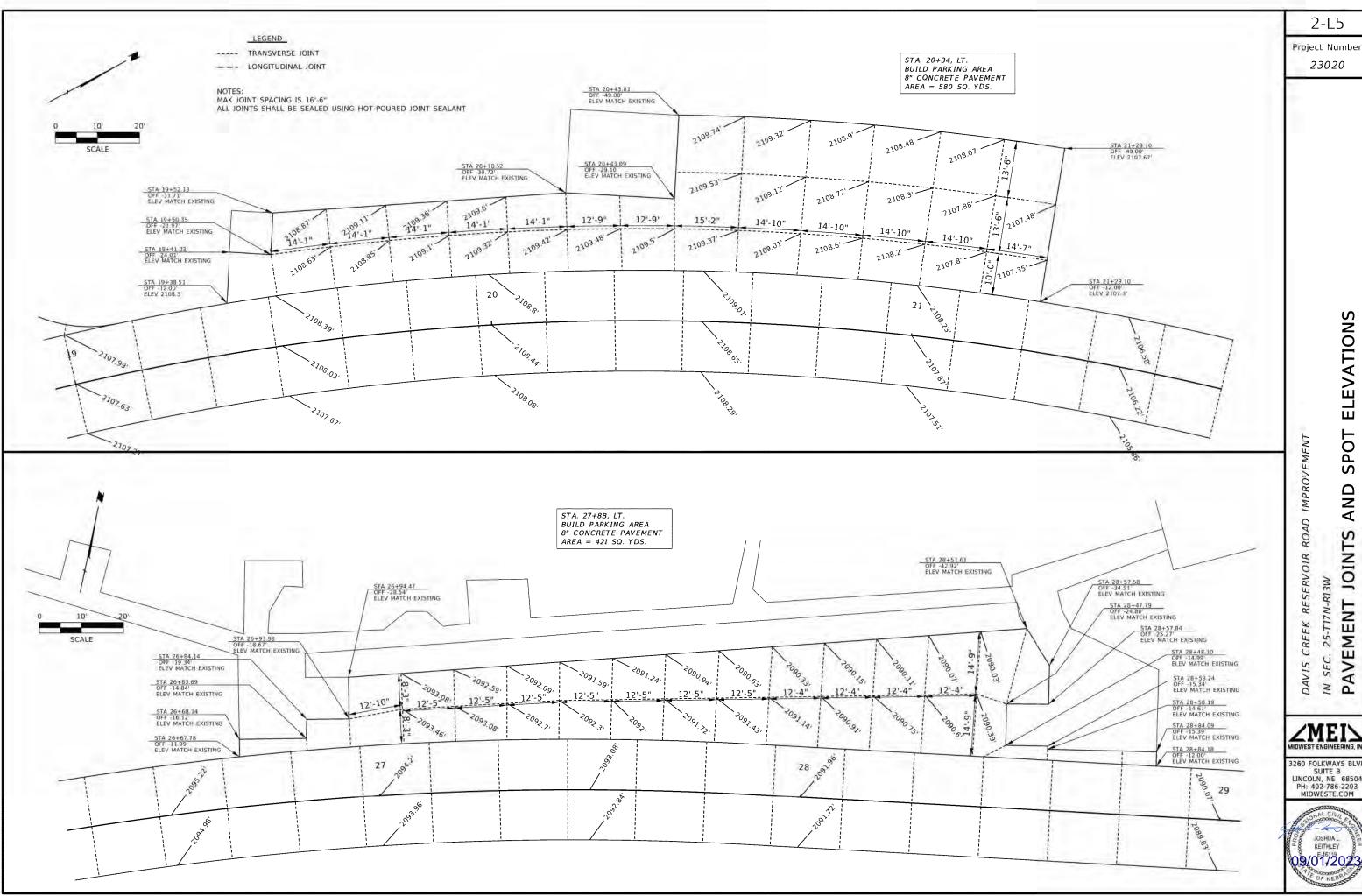
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JOSHUAL. KEITHLEY 09/01/2023

ALL JOINTS SHALL BE SEALED USING HOT-POURED JOINT SEALANT







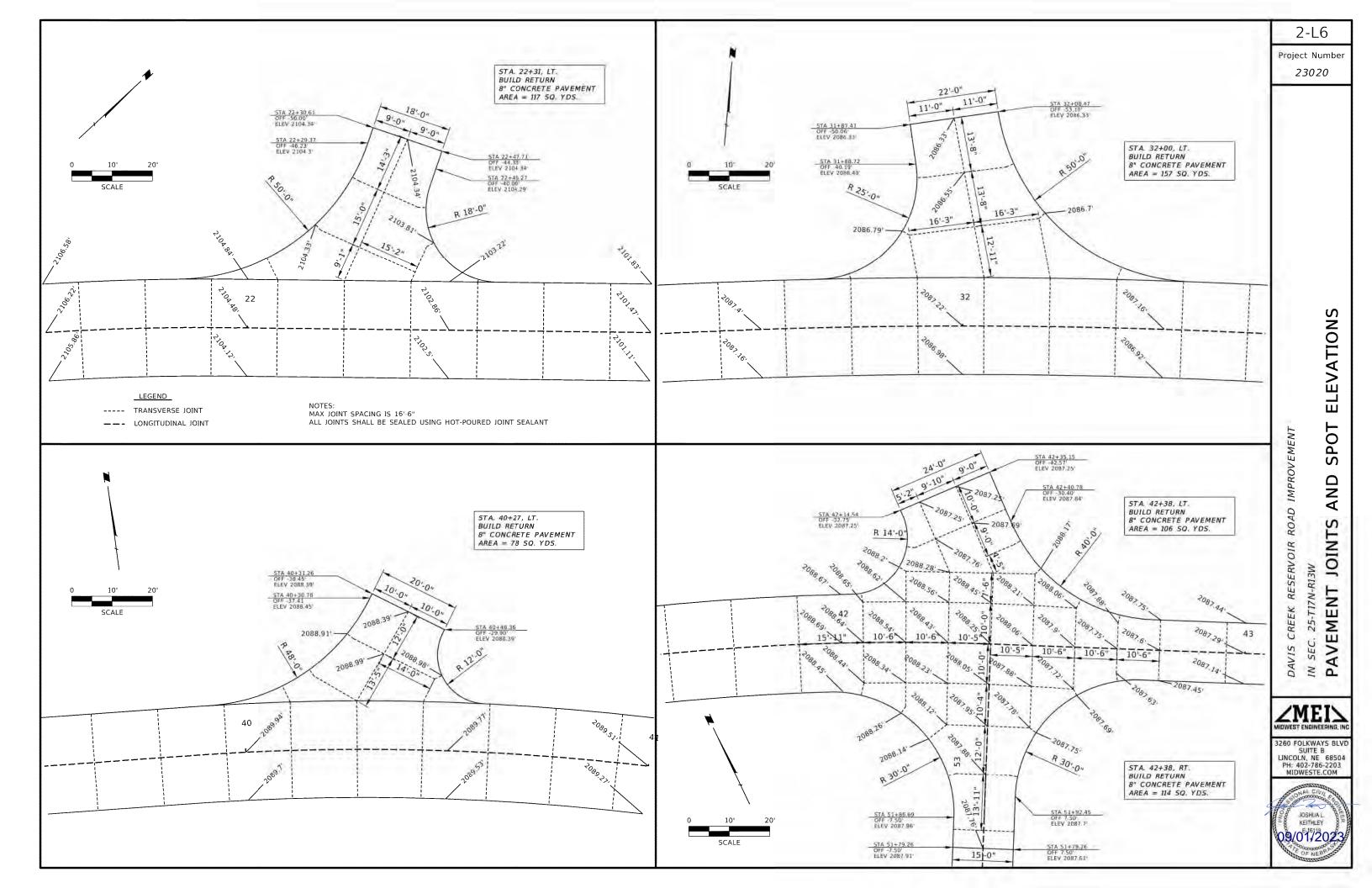
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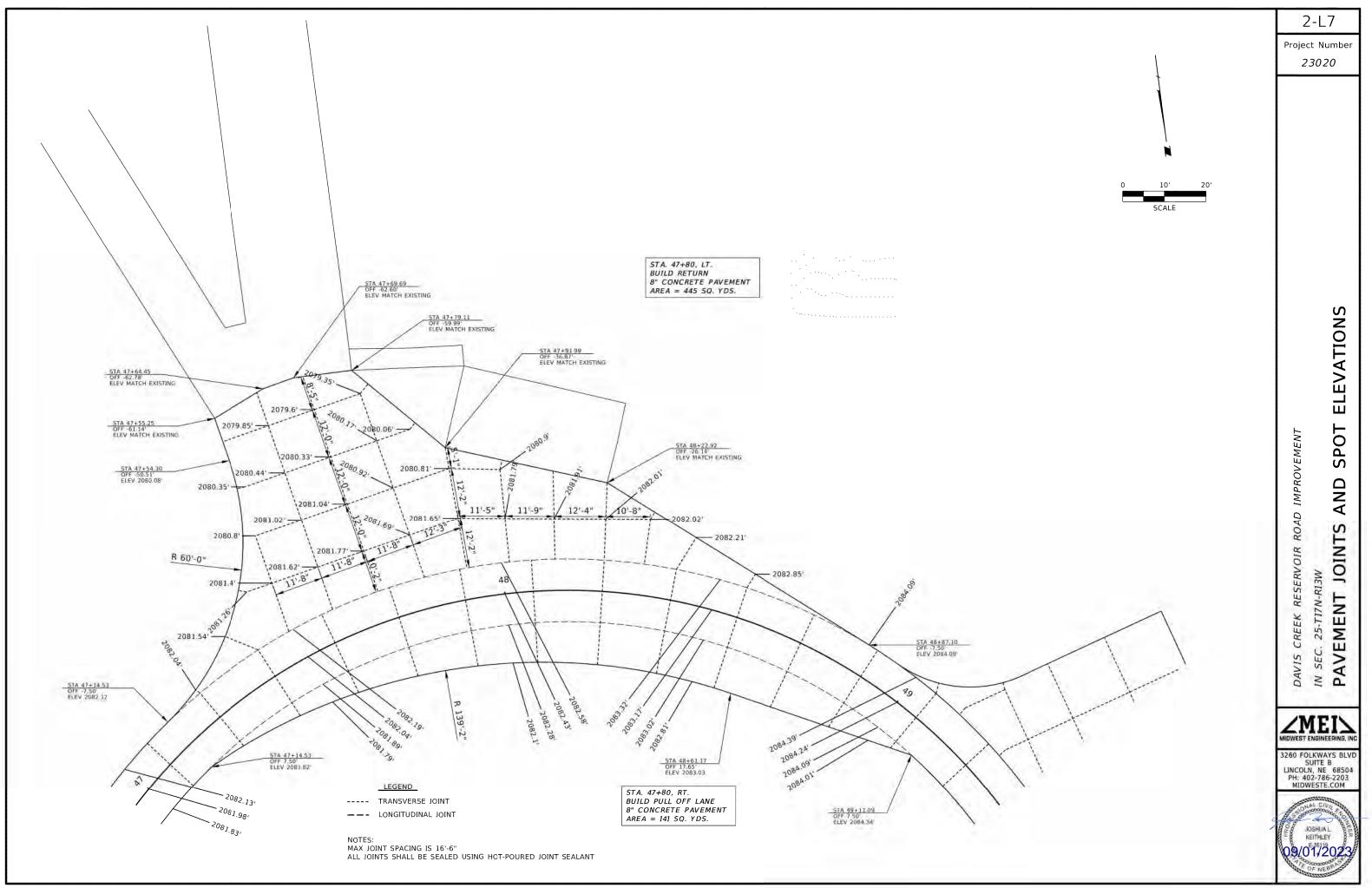
**ELEVATIONS** SPOT AND PAVEMENT JOINTS

MEI

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23020

**ELEVATIONS** SPOT AND

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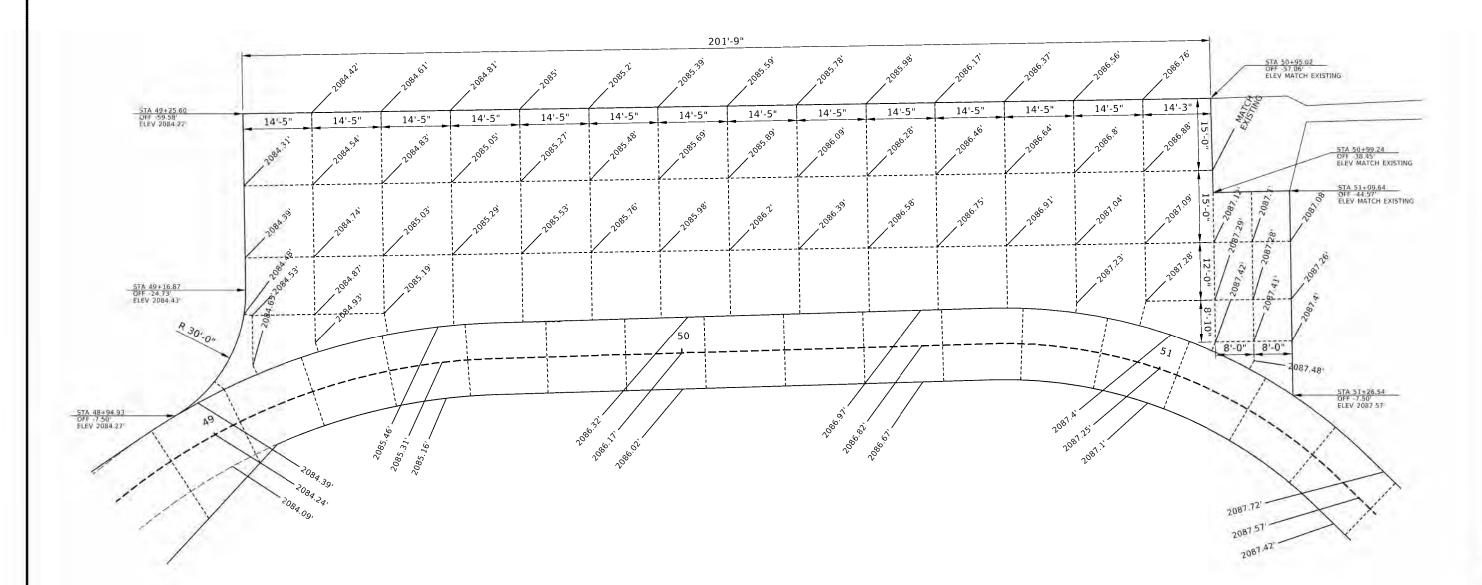
Project Number 23020

**ELEVATIONS** 

SPOT

AND

STA. 50+00, LT.
BUILD PARKING AREA
8" CONCRETE PAVEMENT
AREA = 1,000 SQ. YDS.



LEGEND

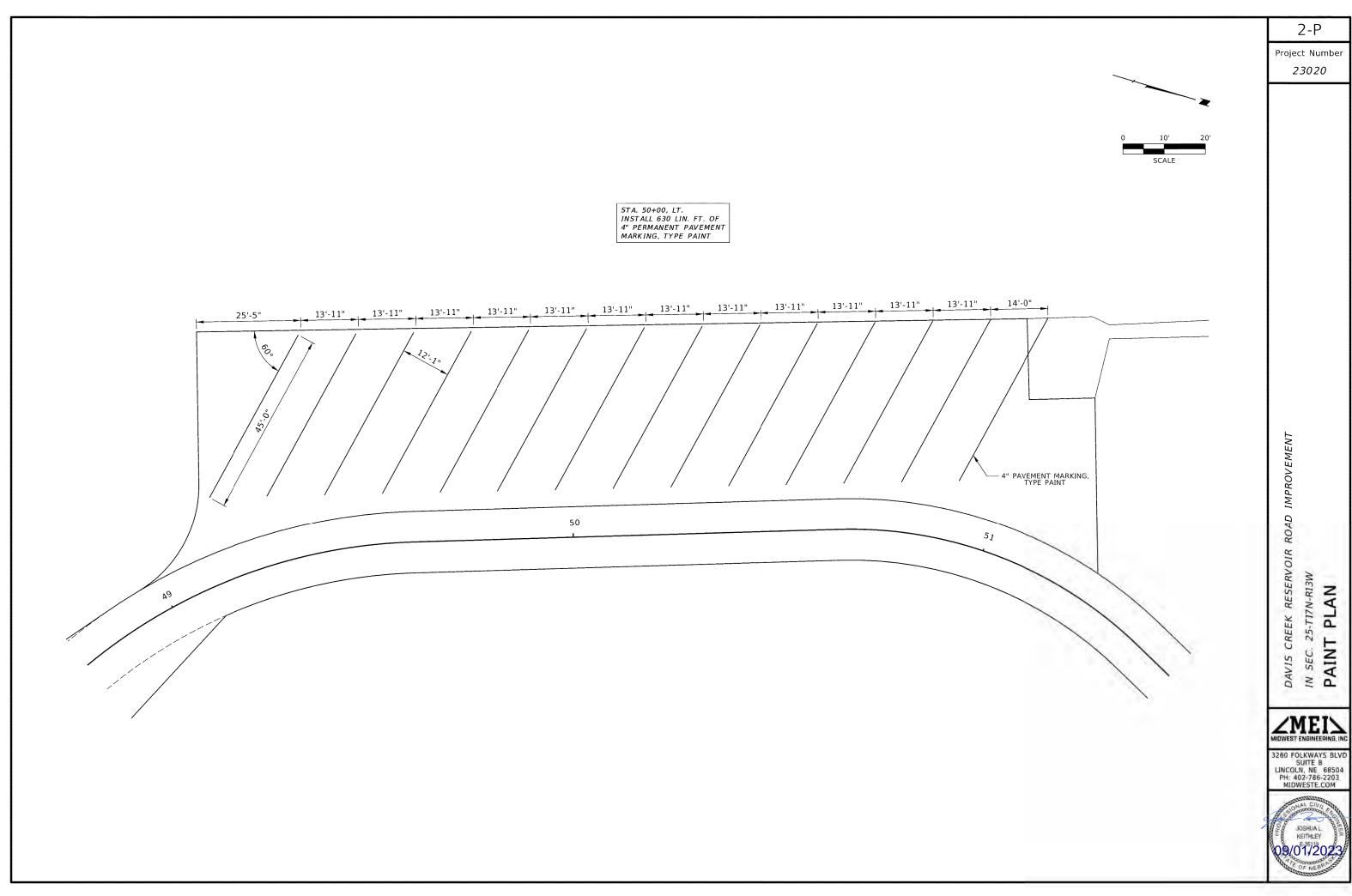
---- TRANSVERSE JOINT

--- LONGITUDINAL JOINT

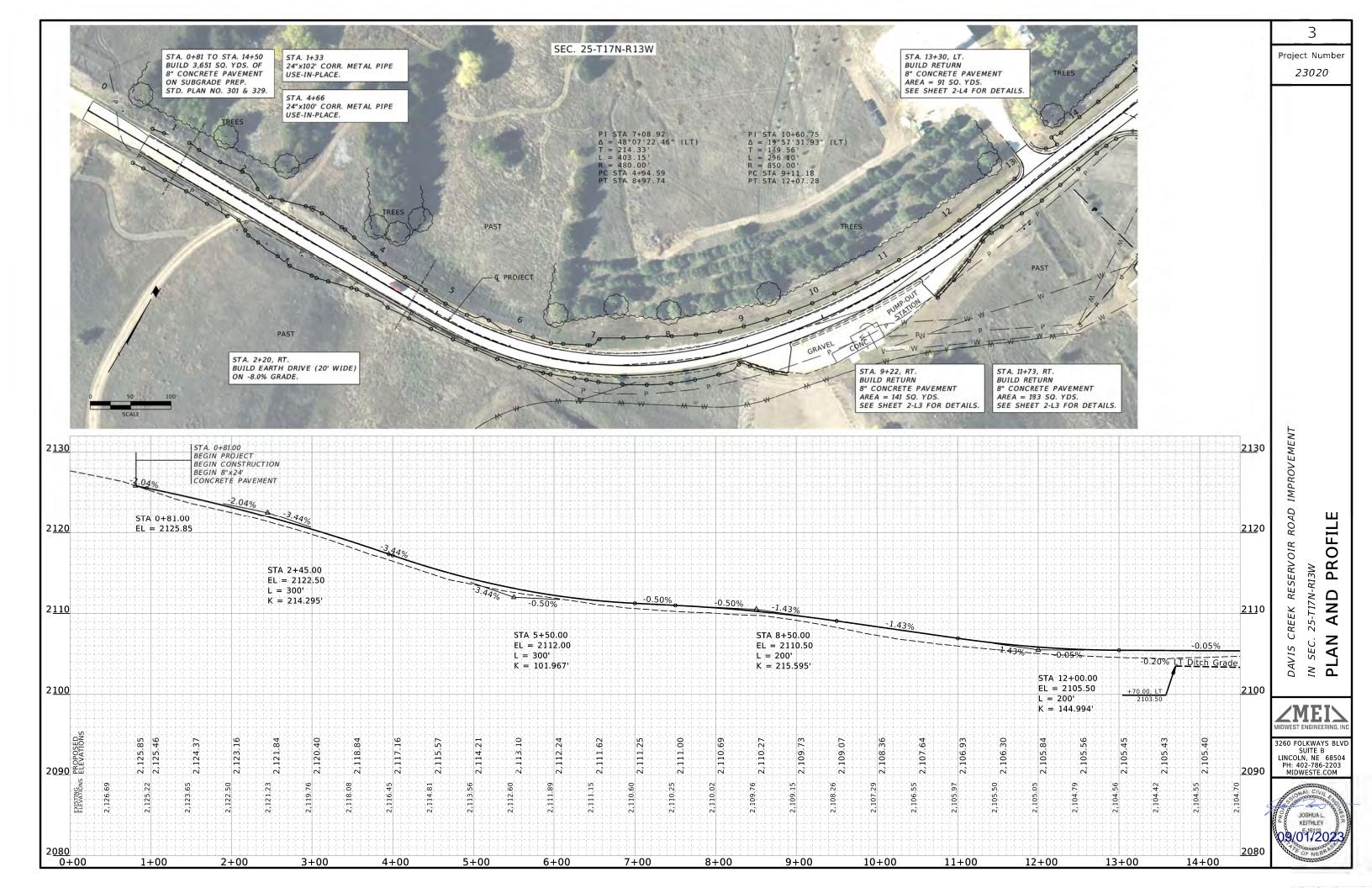
NOTES: MAX JOINT SPACING IS 16'-6"
ALL JOINTS SHALL BE SEALED USING HOT-POURED JOINT SEALANT DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TIZN-RI3W
PAVEMENT JOINTS

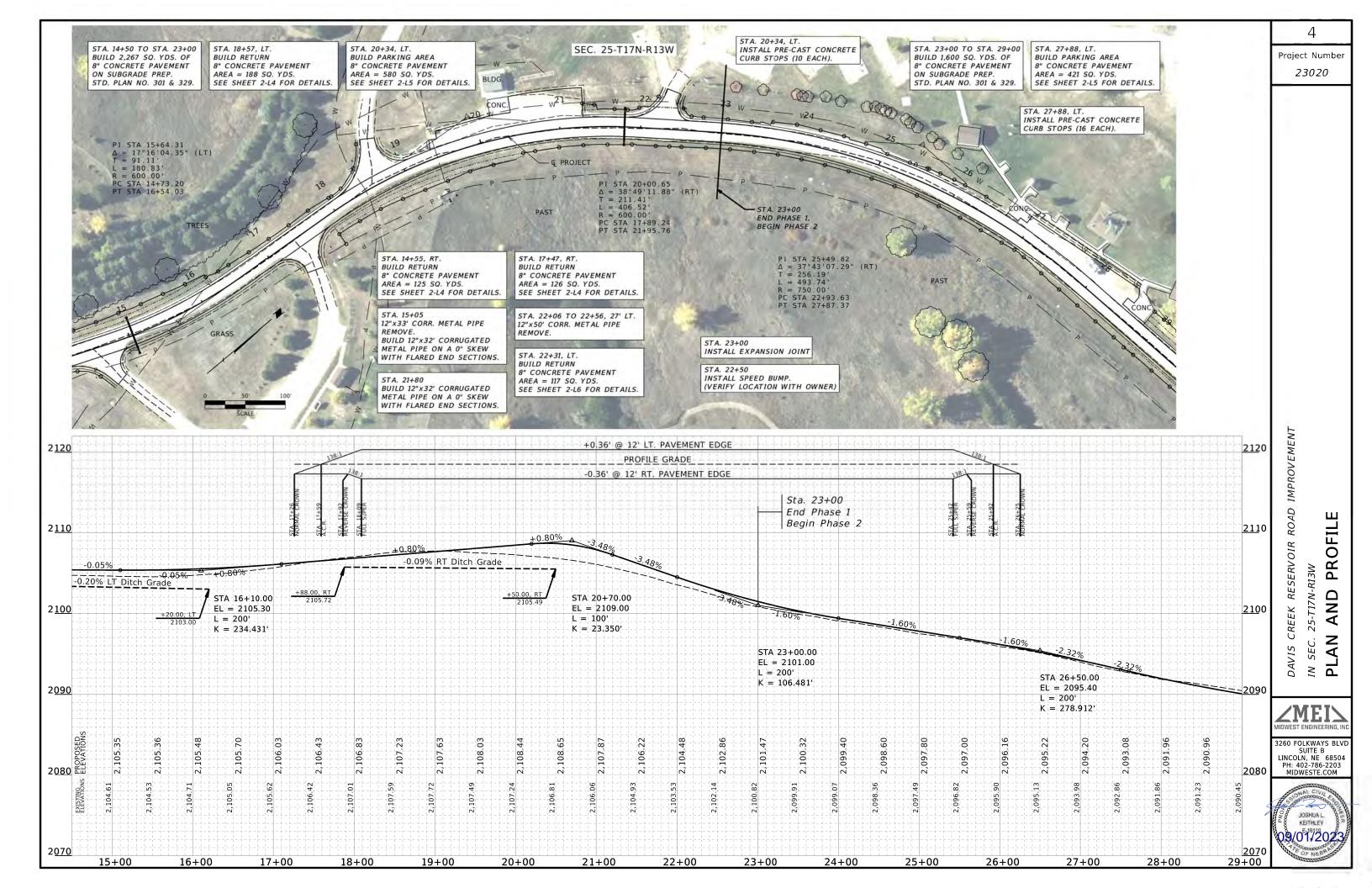
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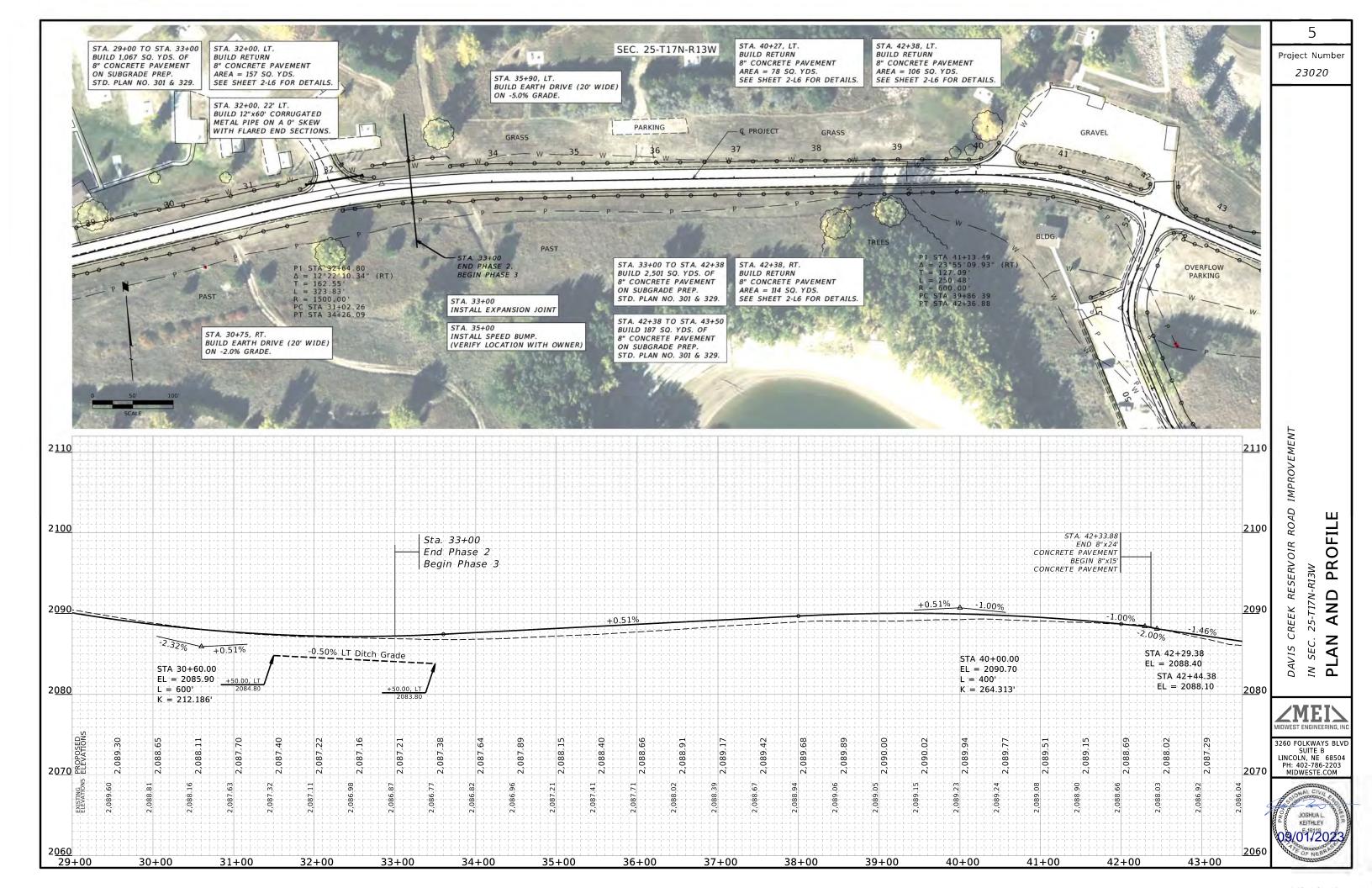


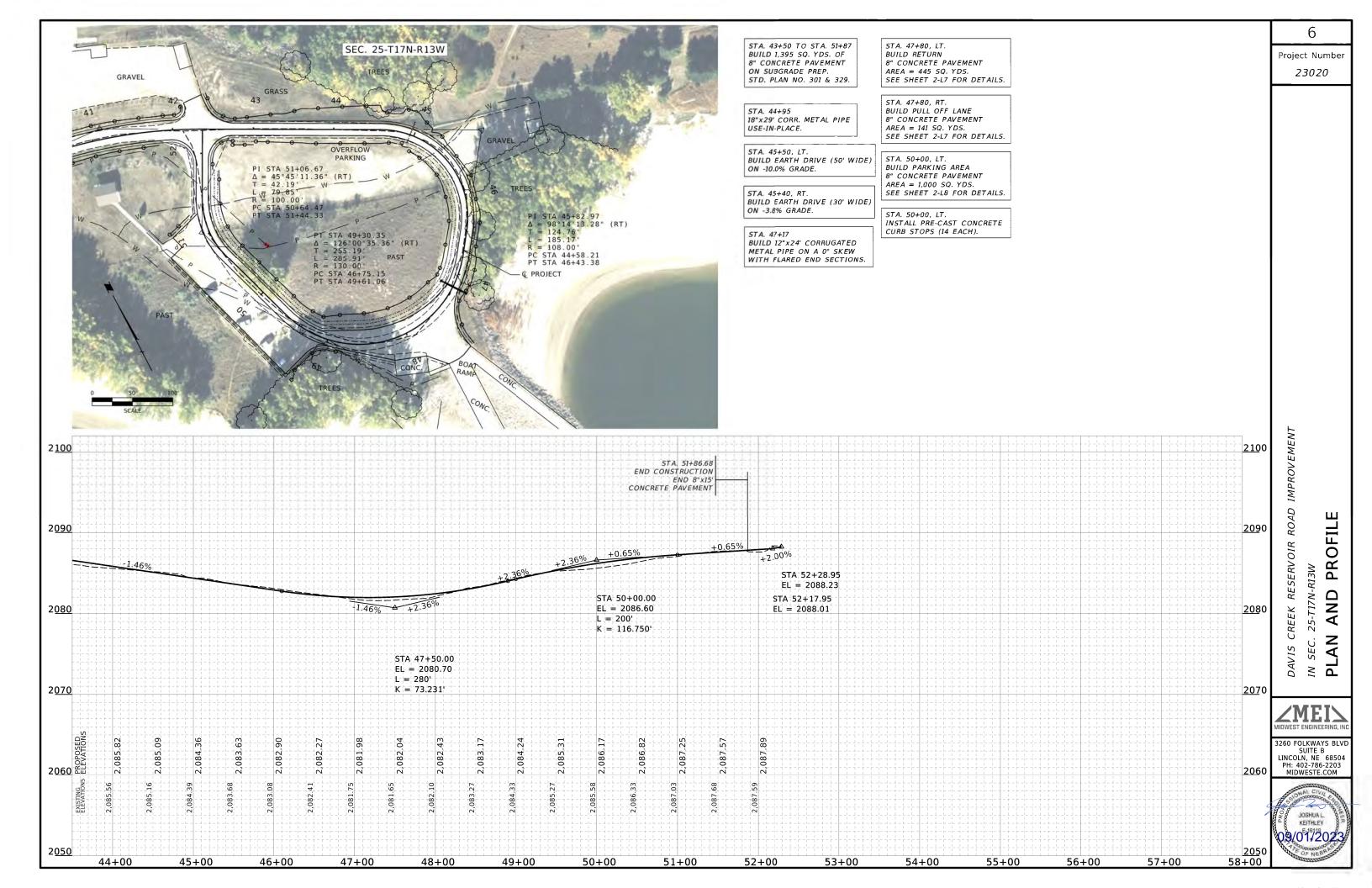






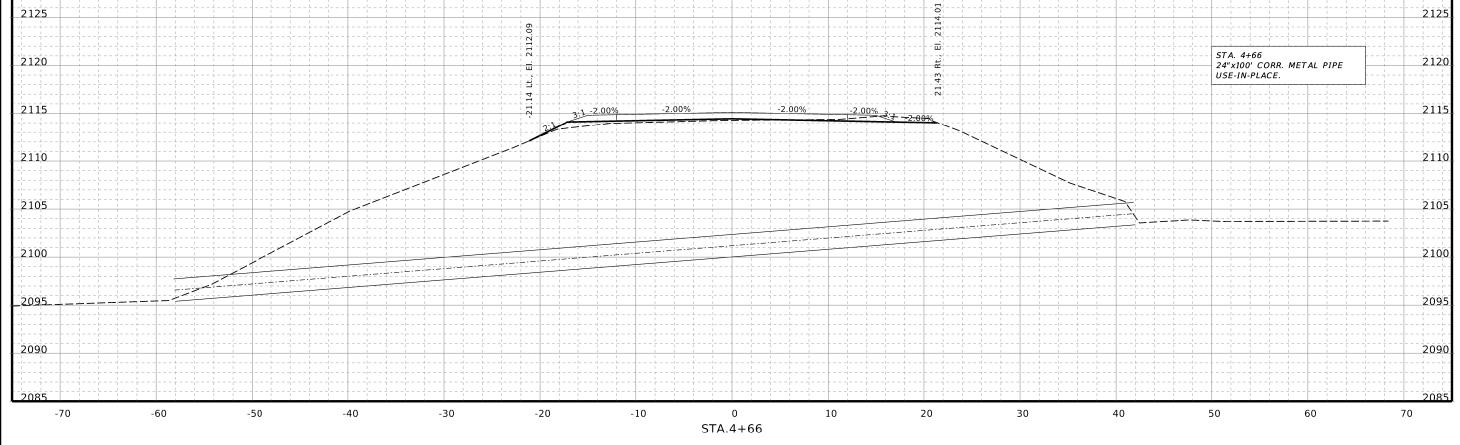


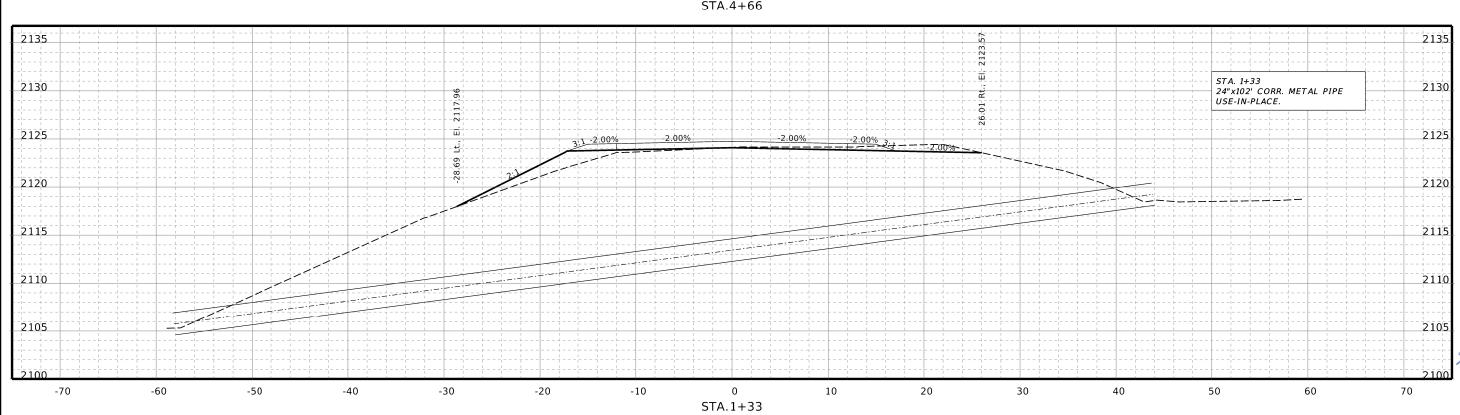




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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-RI3W

CULVERT CROSS SECTIONS

MEI
MIDWEST ENGINEERING, INC



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CREEK RESERVOIR ROAD IMPROVEMENT

SECTIONS

IN SEC. 25-TI7N-RI3W

CULVERT CROSS

MEI
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3260 FOLKWAYS BLVD
SUITE R

3260 FOLKWAYS BLVD SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM

2105

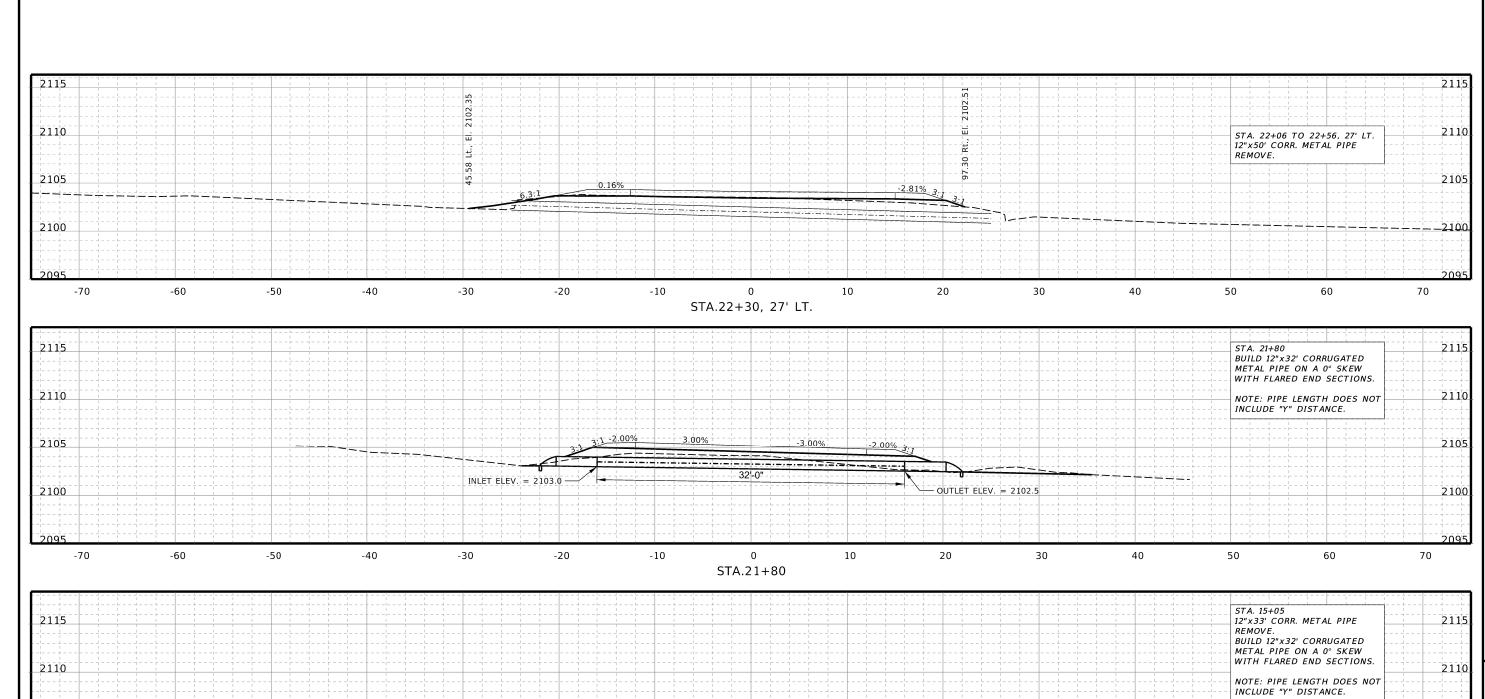
2100

70

60

50





-20

-10

0

STA.15+05

10

20

-40

-30

2105

2100

-70

40

50

2095

2090

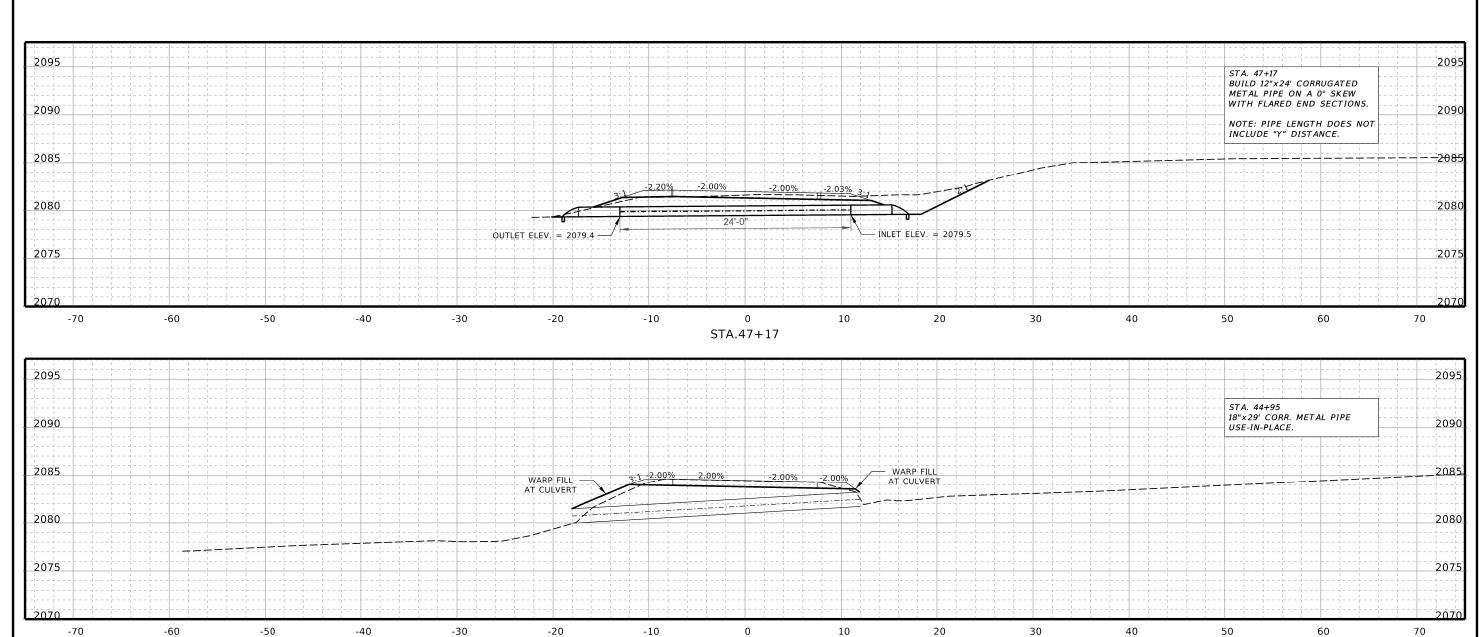
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STA. 32+00, 22' LT.
BUILD 12"x60' CORRUGATED
METAL PIPE ON A 0° SKEW
WITH FLARED END SECTIONS.

NOTE: PIPE LENGTH DOES NOT

60

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2095

2090

-70

-40

-30

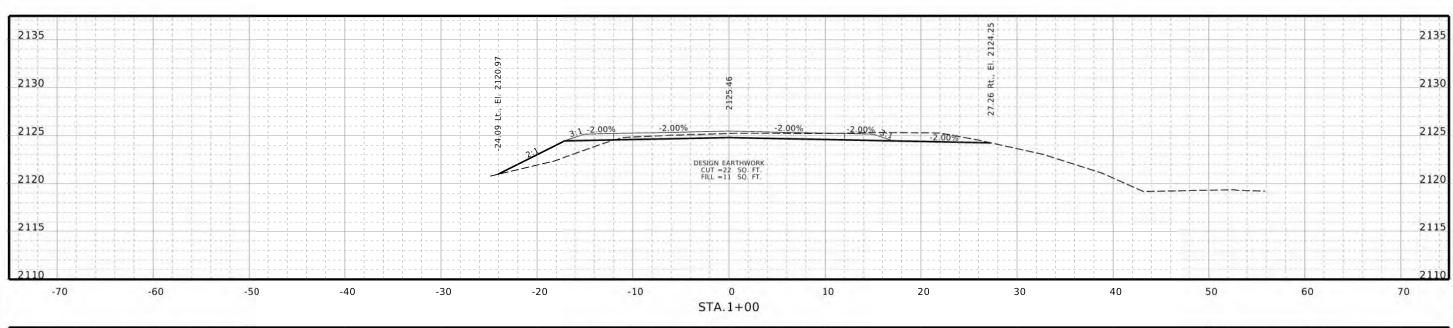
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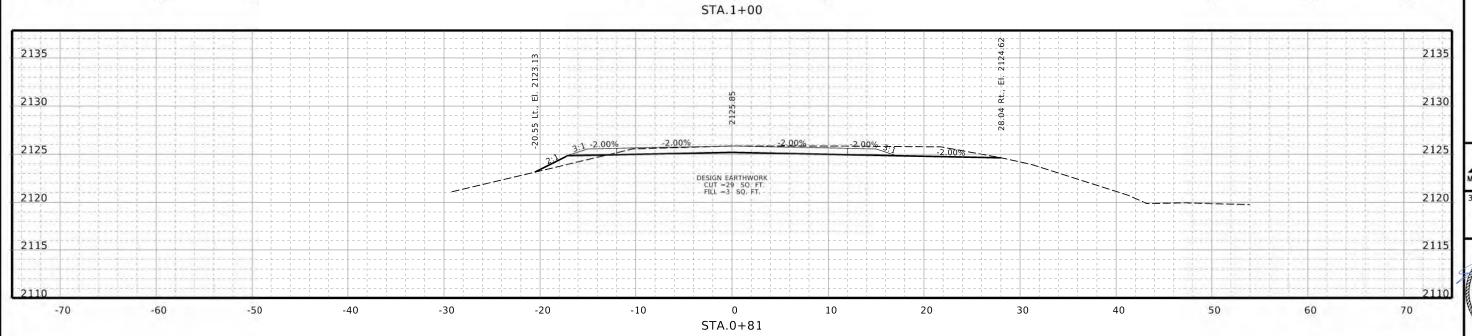
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STA.32+00, 22' LT.

10

20



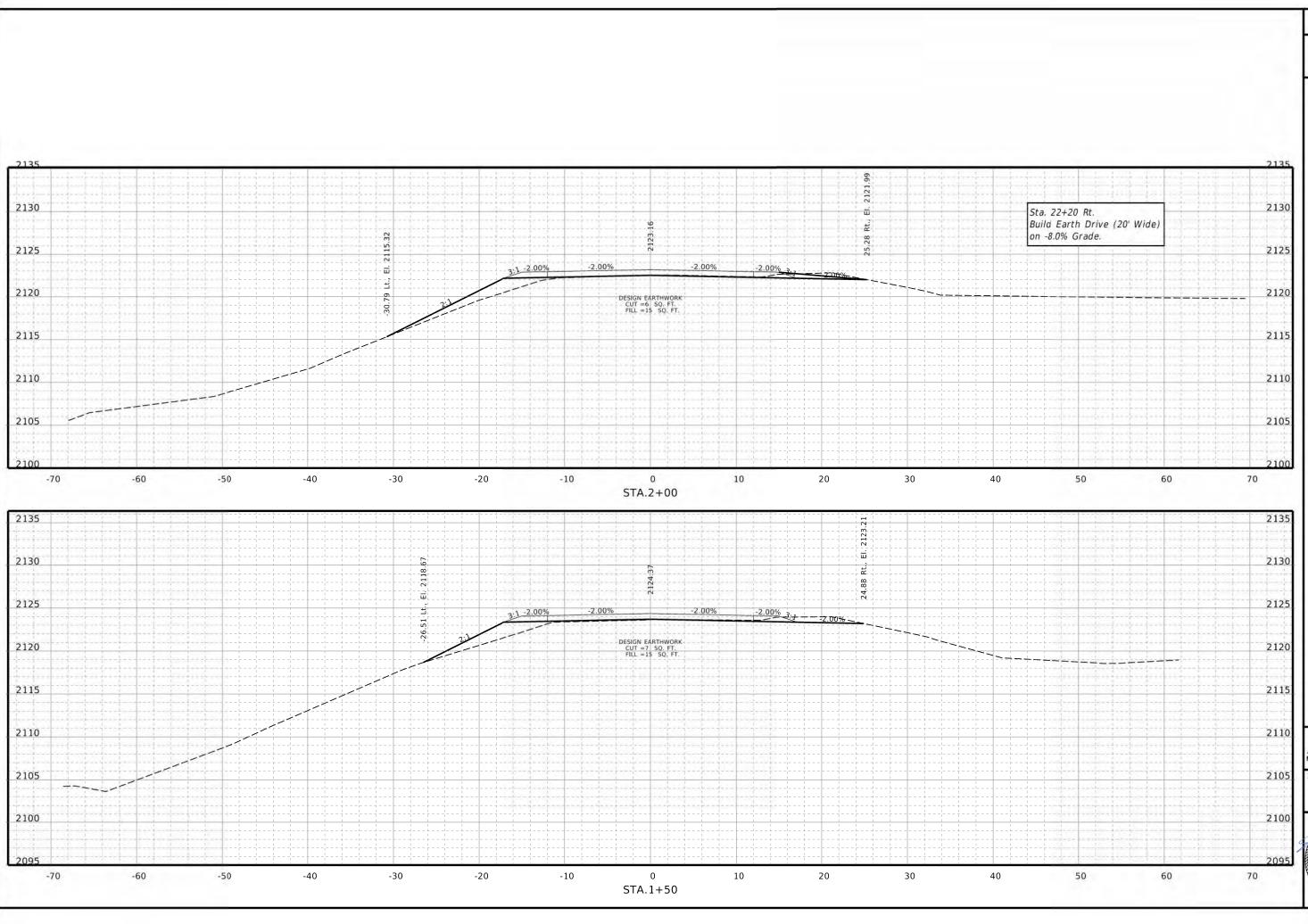


DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-T17N-R13W

CROSS SECTIONS

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MIDWEST ENGINEERING, INC
3260 FOLKWAYS BLVD





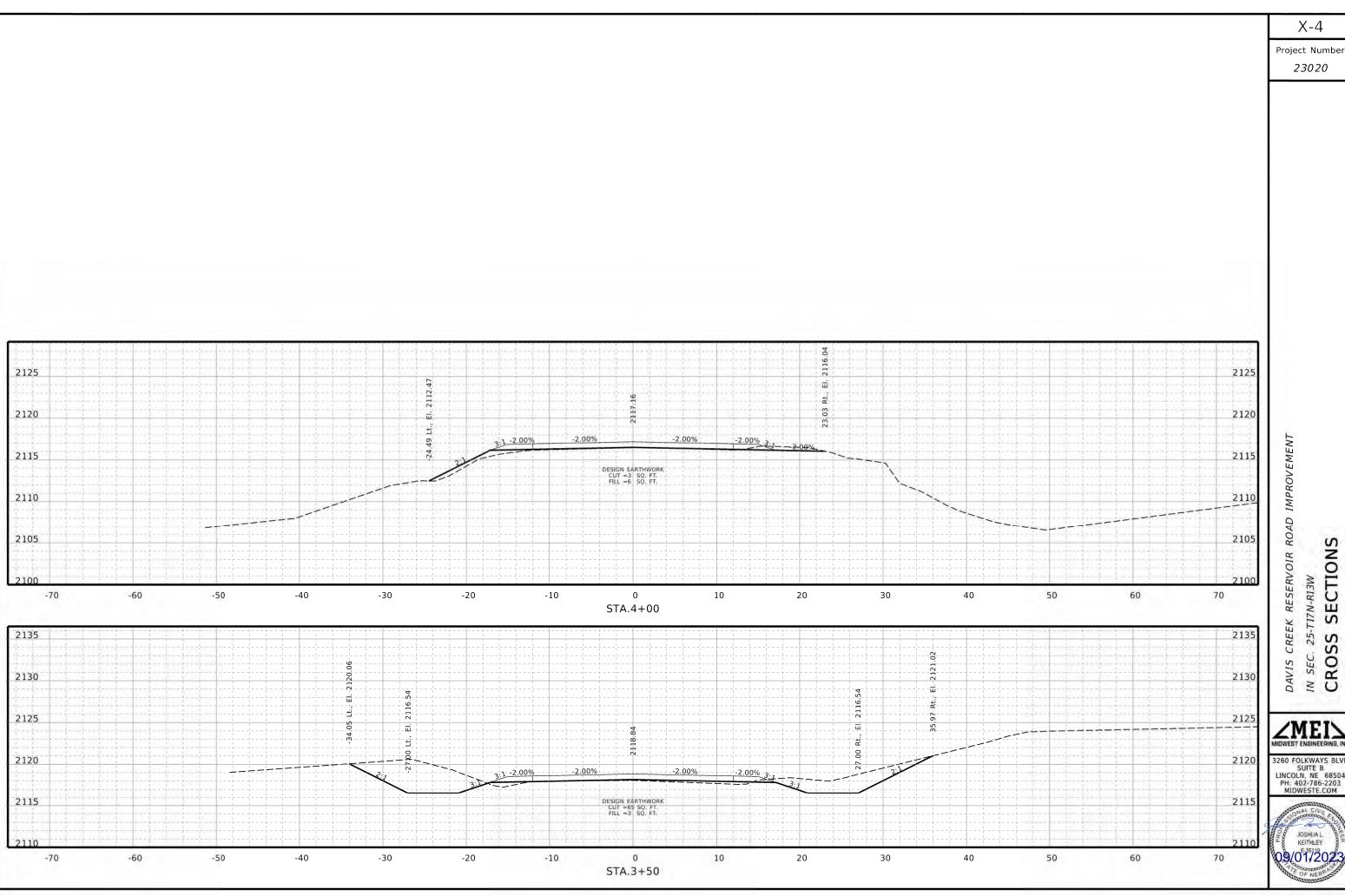
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CROSS SECTIONS

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MIDWEST ENGINEERING, INC







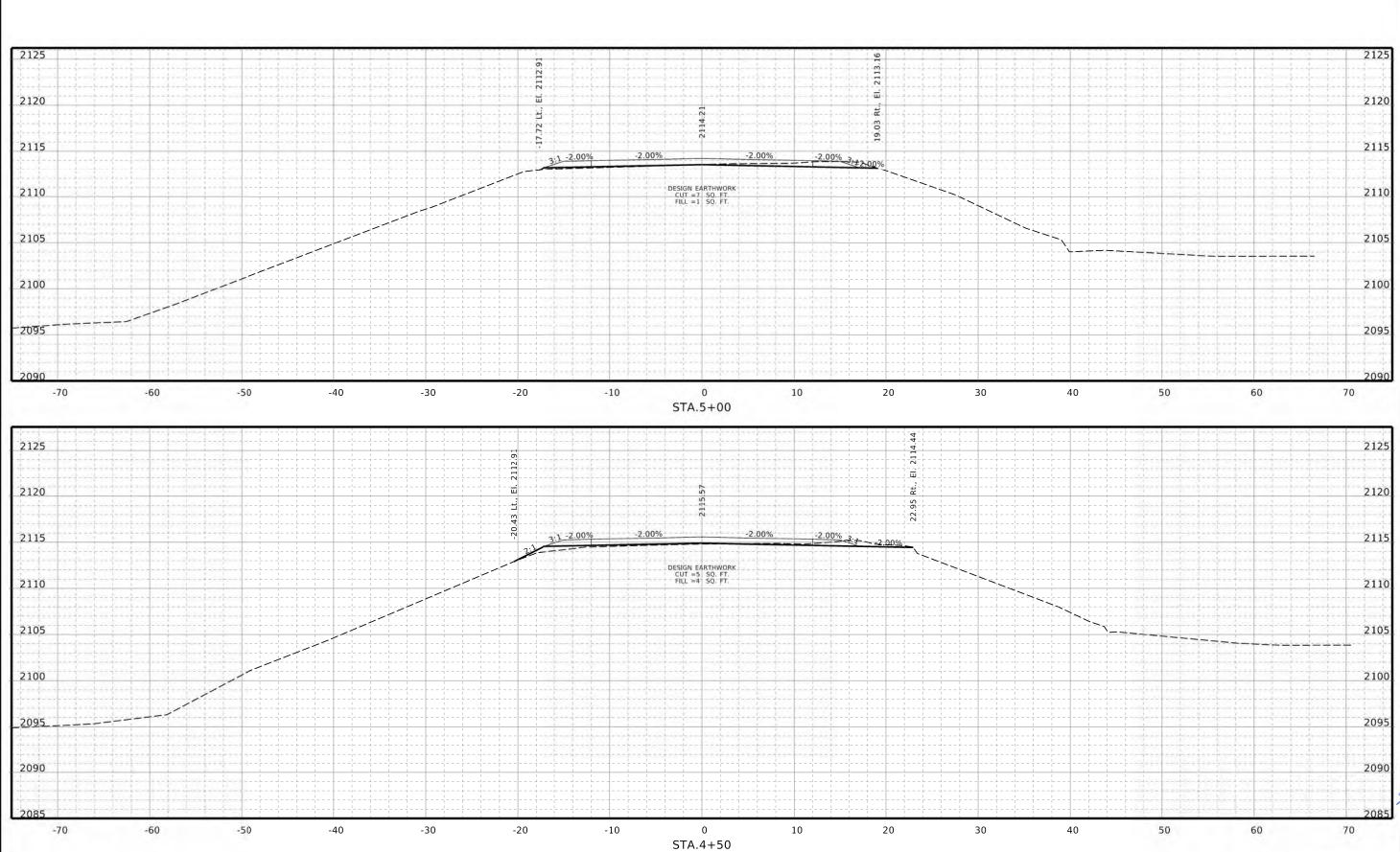
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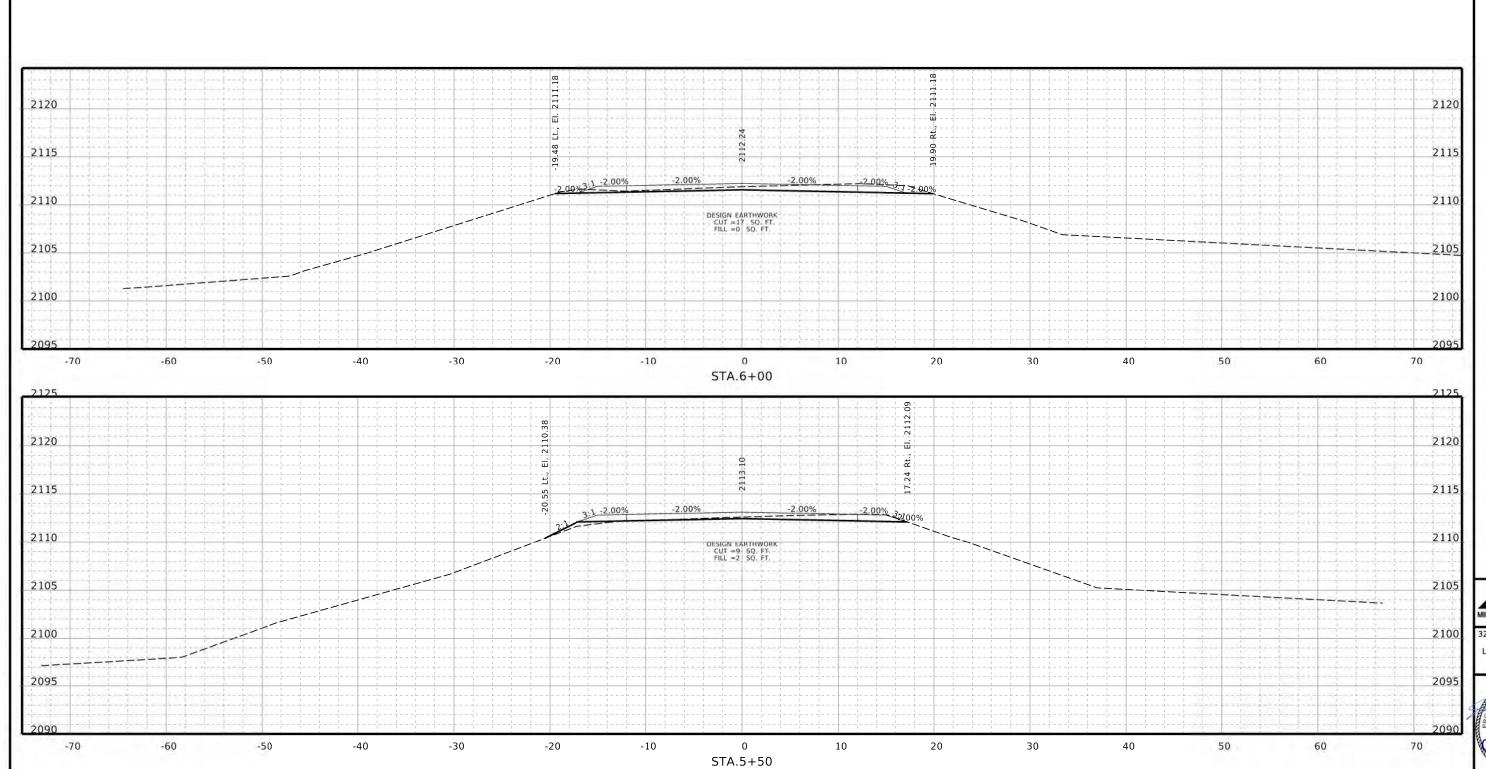
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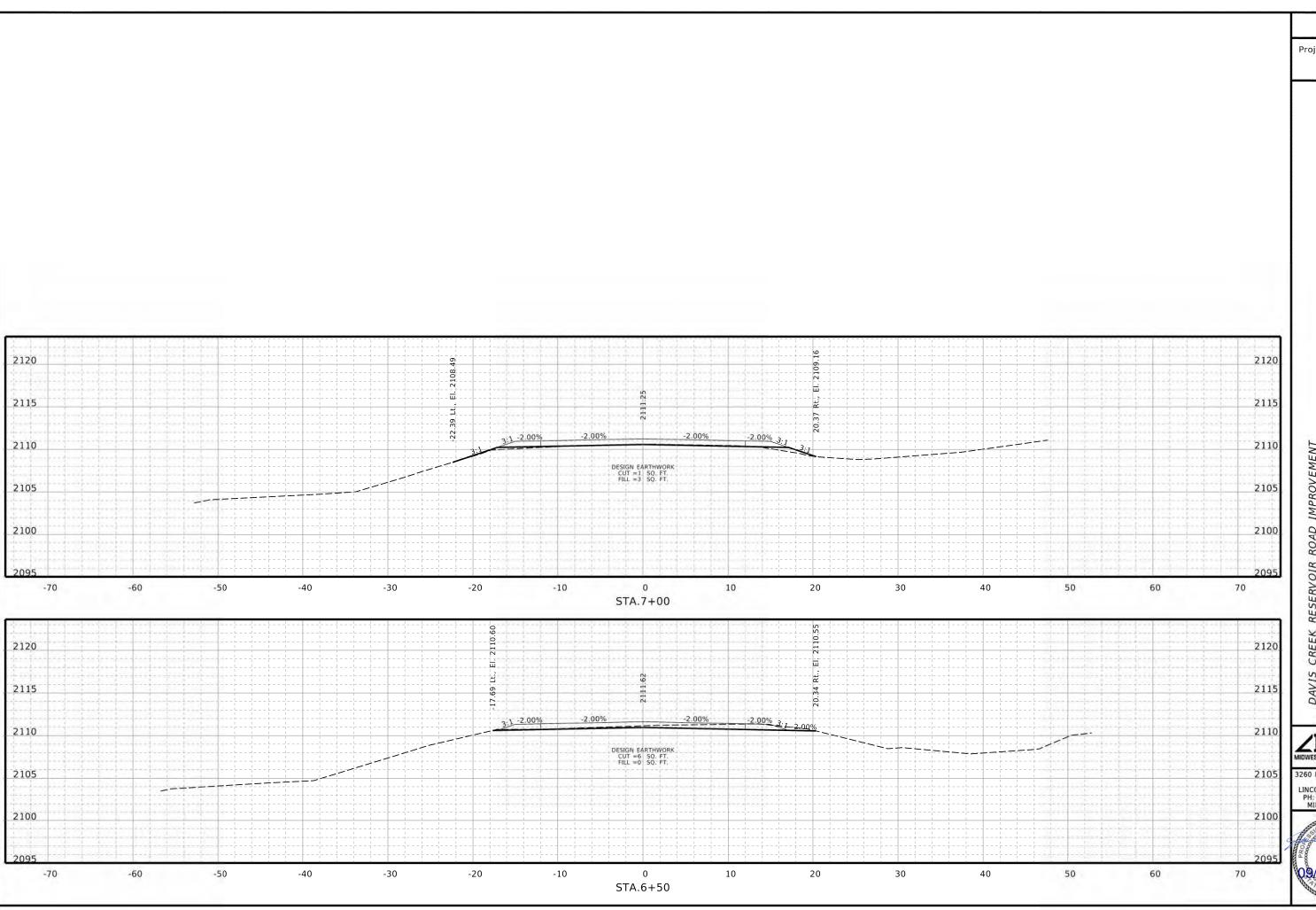




DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W
CROSS SECTIONS







Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W

CROSS SECTIONS

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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W
CROSS SECTIONS

MEIN MIDWEST ENGINEERING, INC

2110

2100

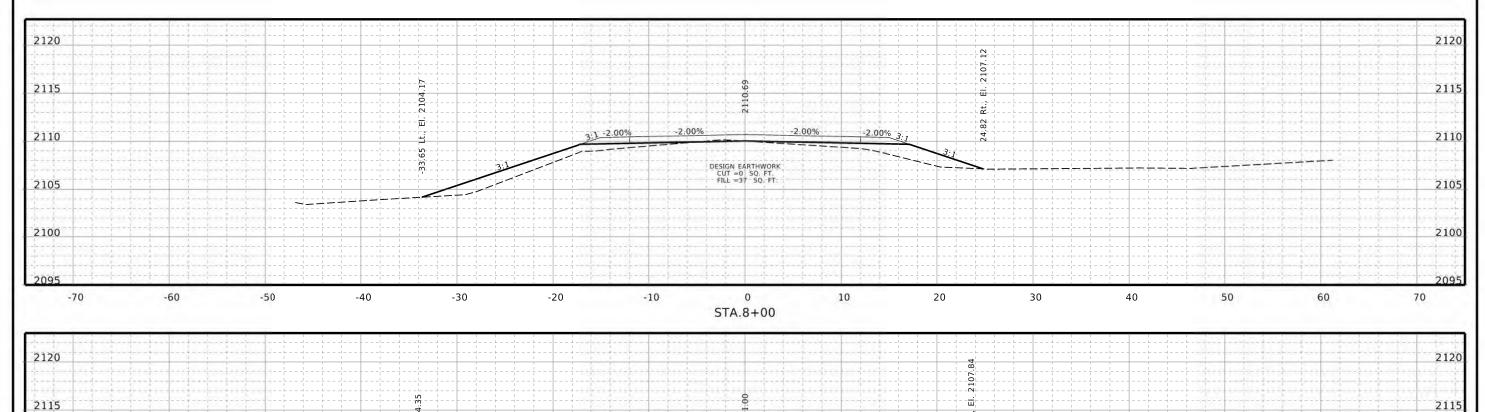
70

50

60

MIDWEST ENGINEERING, INC 3260 FOLKWAYS BLVD SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM





0

STA.7+50

20

2110

2105

2100

-60

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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W
CROSS SECTIONS

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Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W

CROSS SECTIONS

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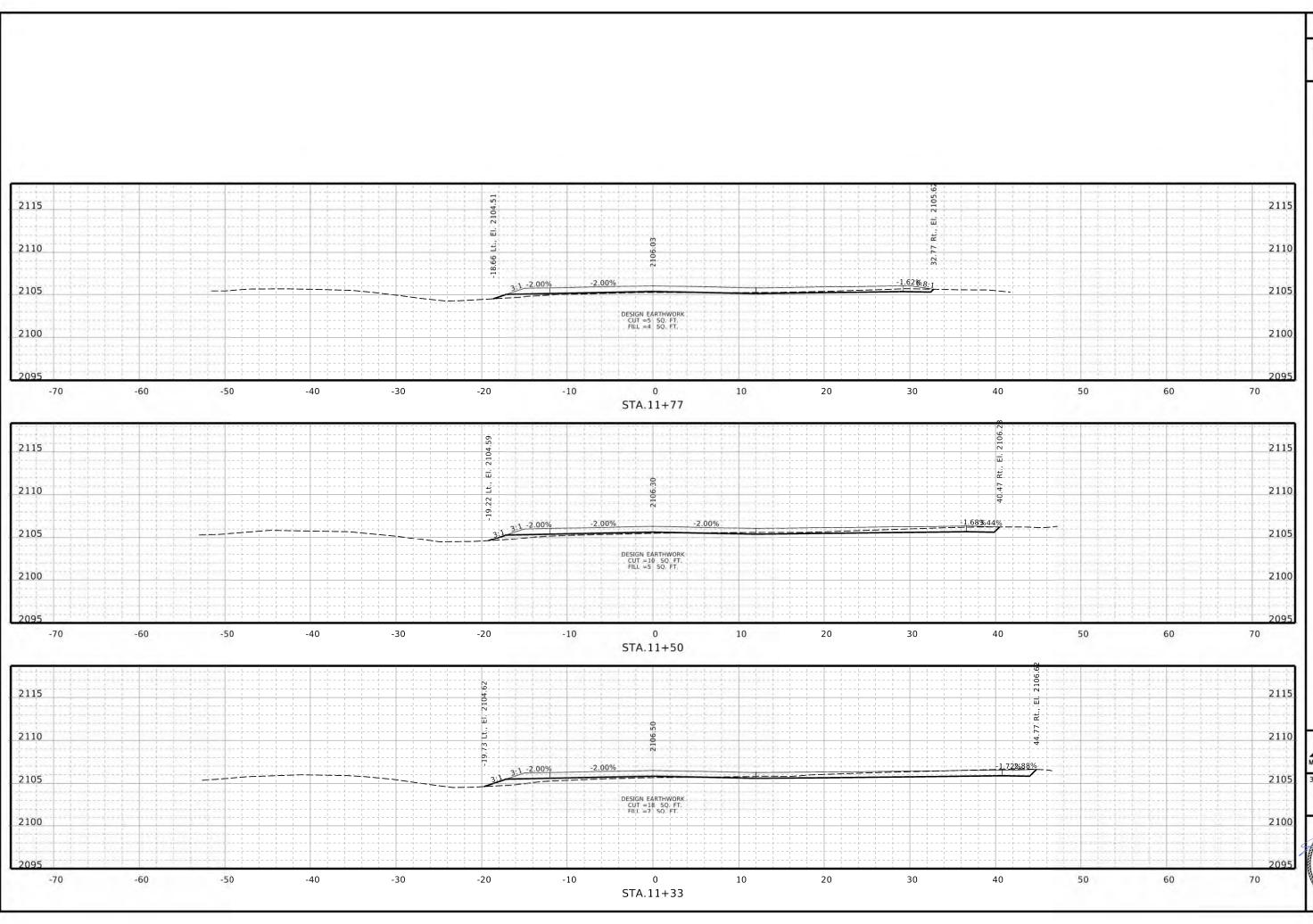
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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-R13W

CROSS SECTIONS

MEI MIDWEST ENGINEERING, INC





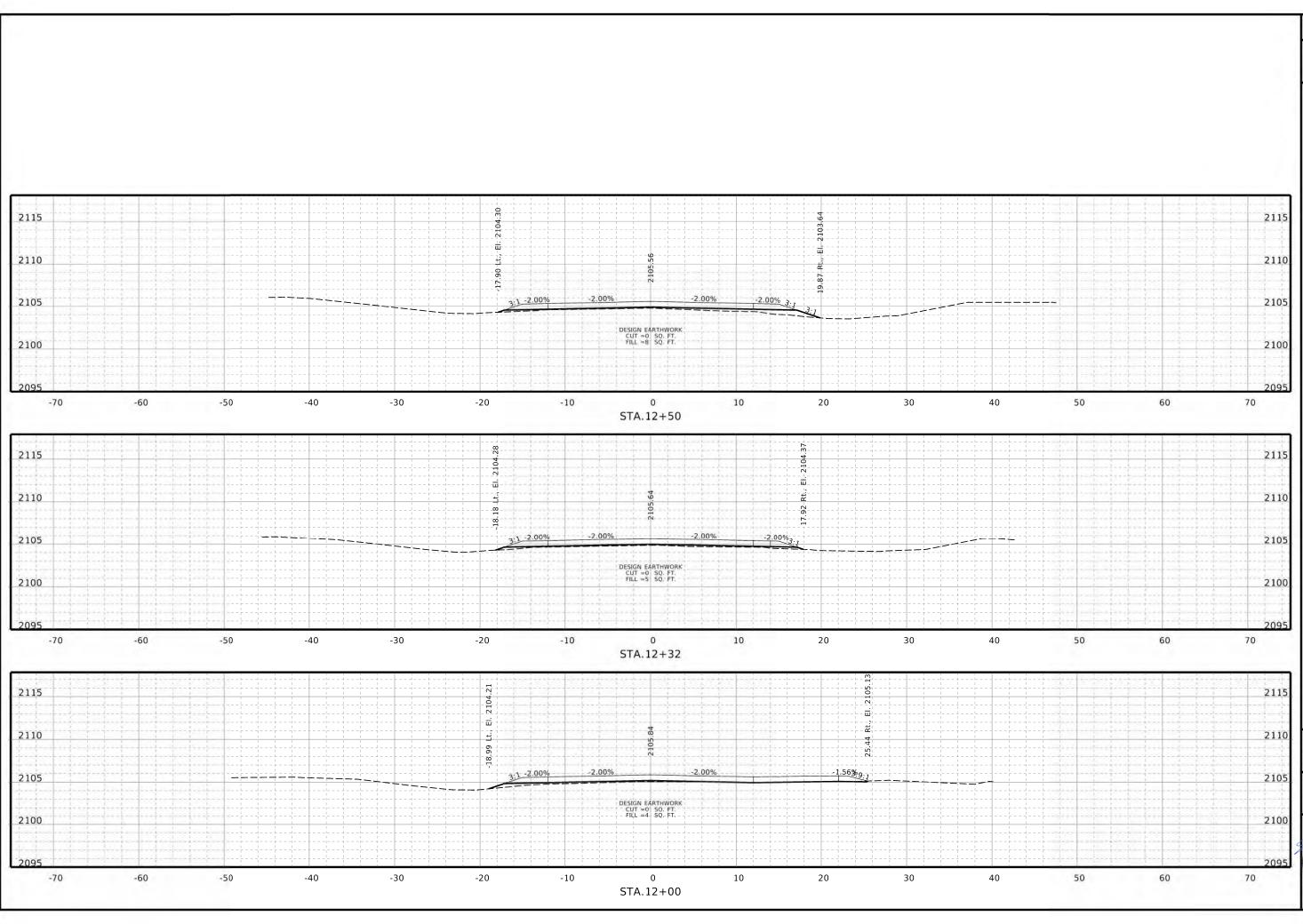
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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TIZN-R13W

CROSS SECTIONS

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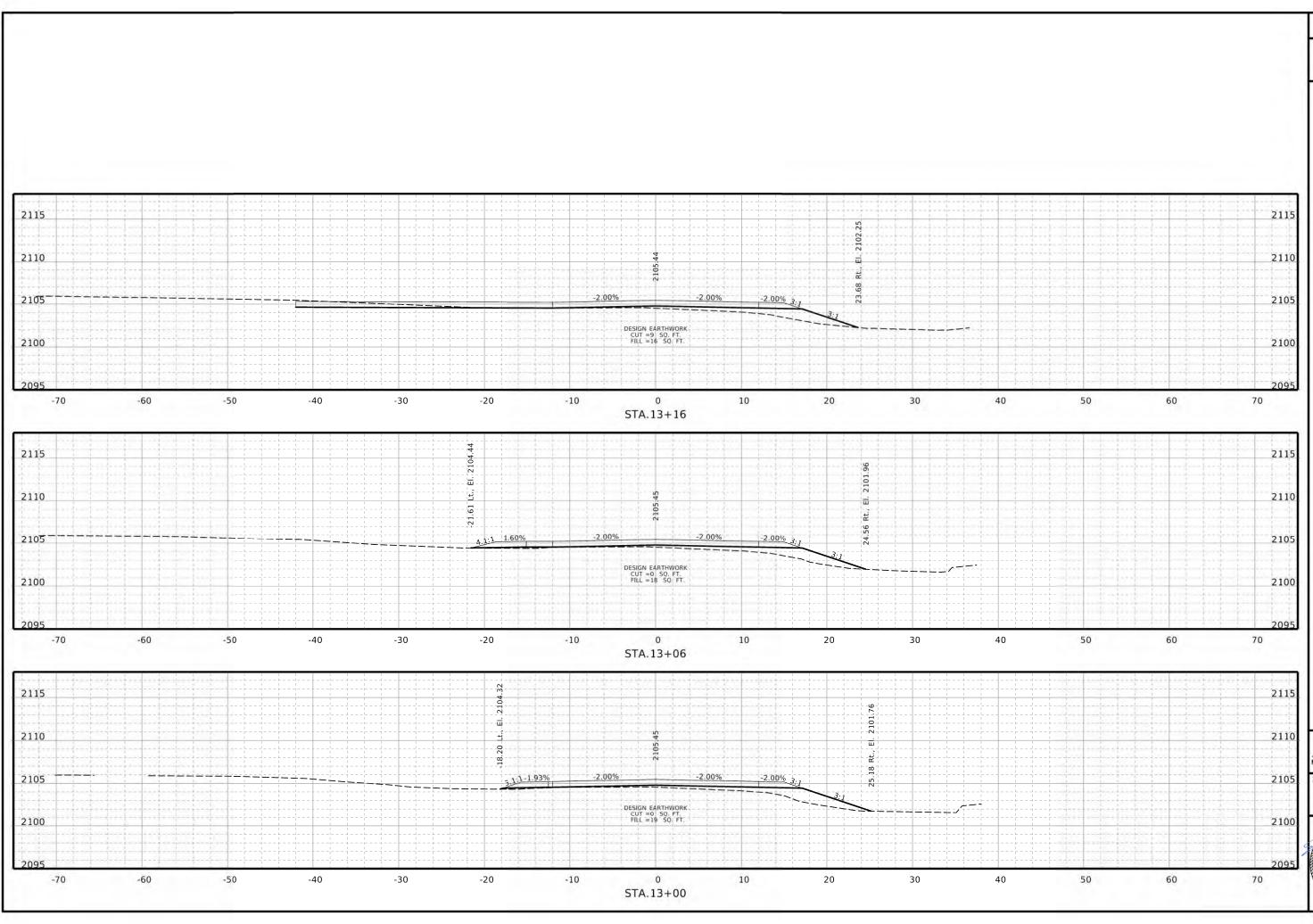
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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TIZN-R13W

CROSS SECTIONS

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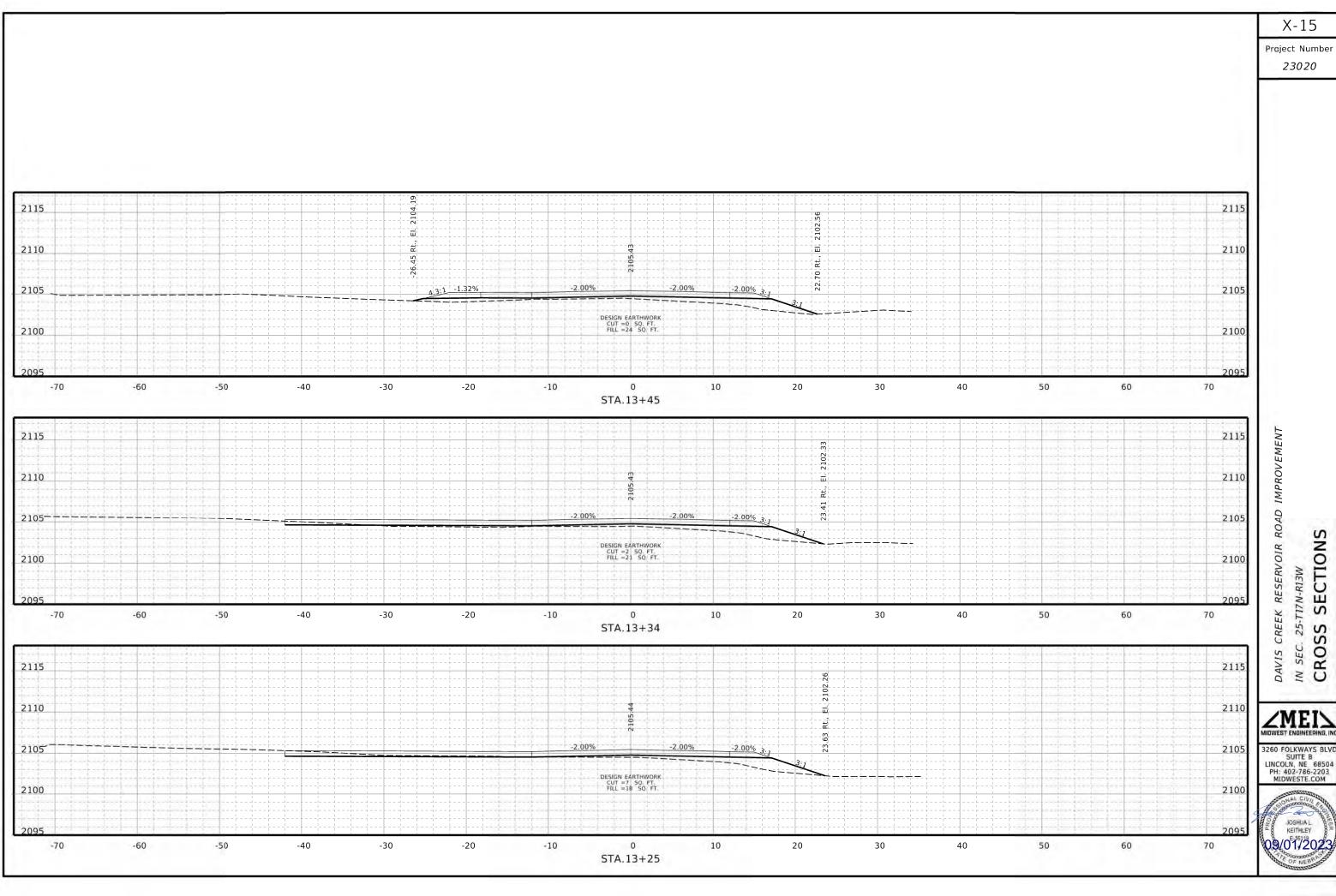


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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W
CROSS SECTIONS

MEIS MIDWEST ENGINEERING, INC





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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W
CROSS SECTIC

SECTIONS

Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-RI3W
CROSS SECTIC

SECTIONS



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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W
CROSS SECTIONS

MEI\
MIDWEST ENGINEERING, INC



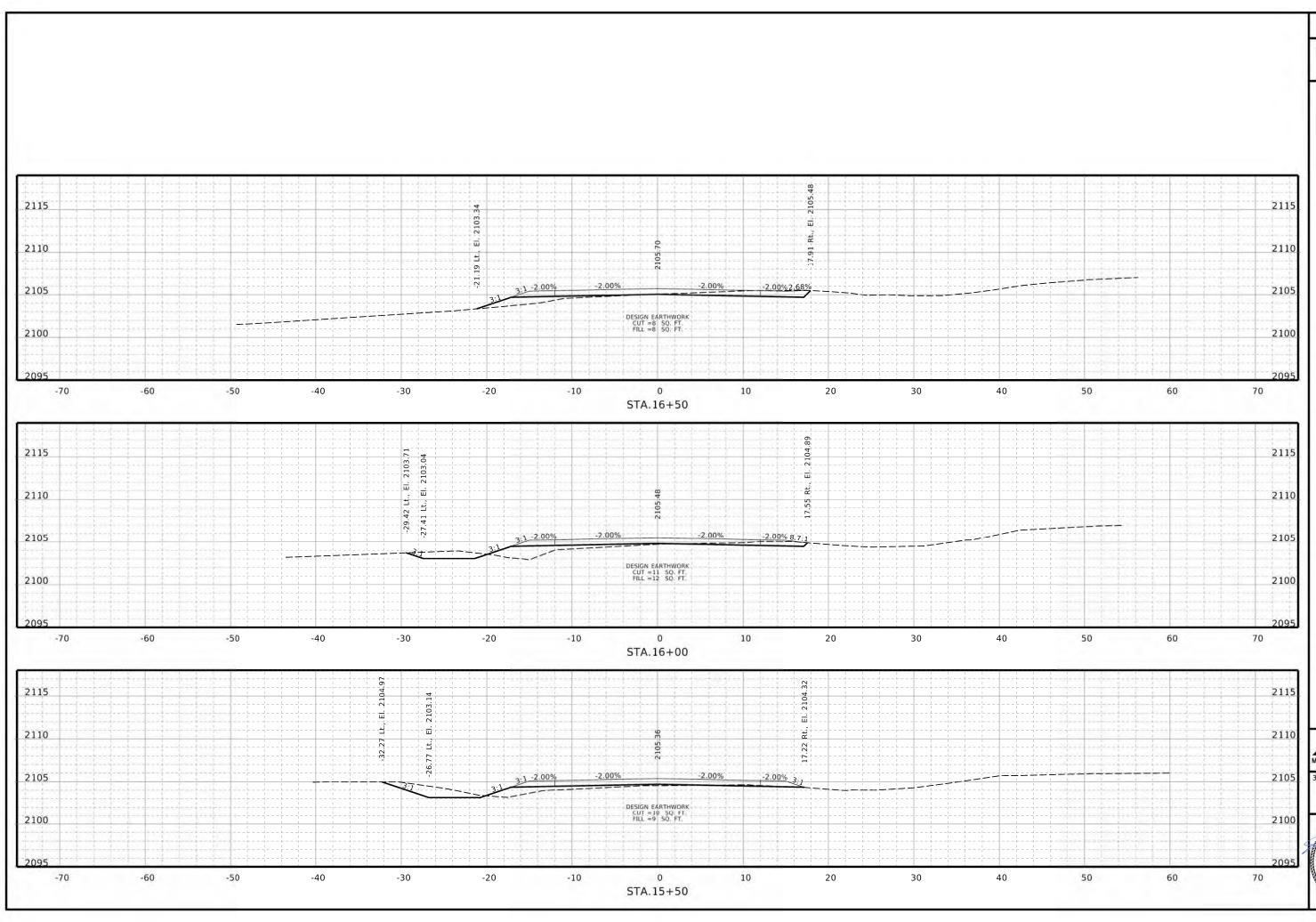
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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-R13W

CROSS SECTIONS

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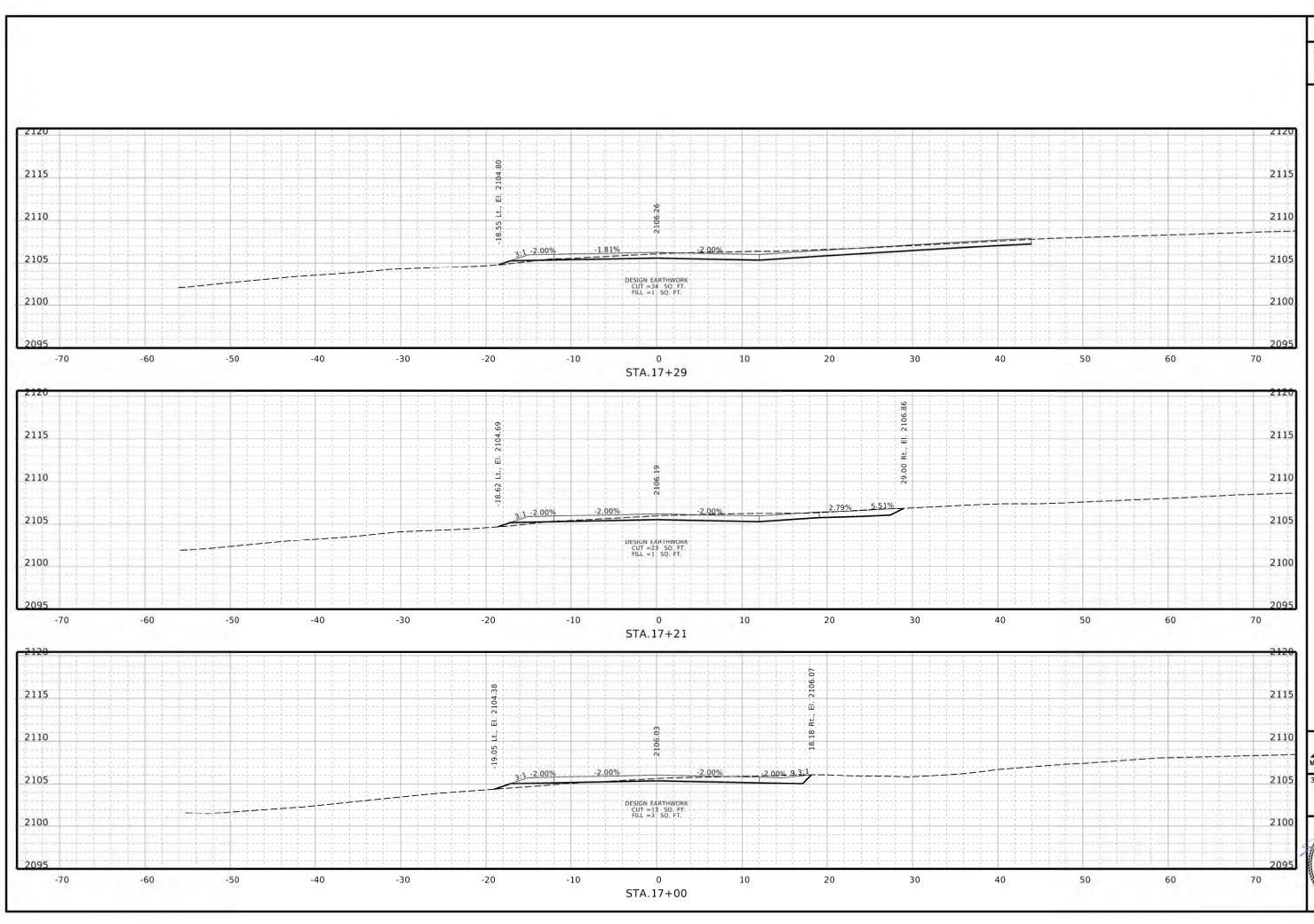


Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W
CROSS SECTIONS

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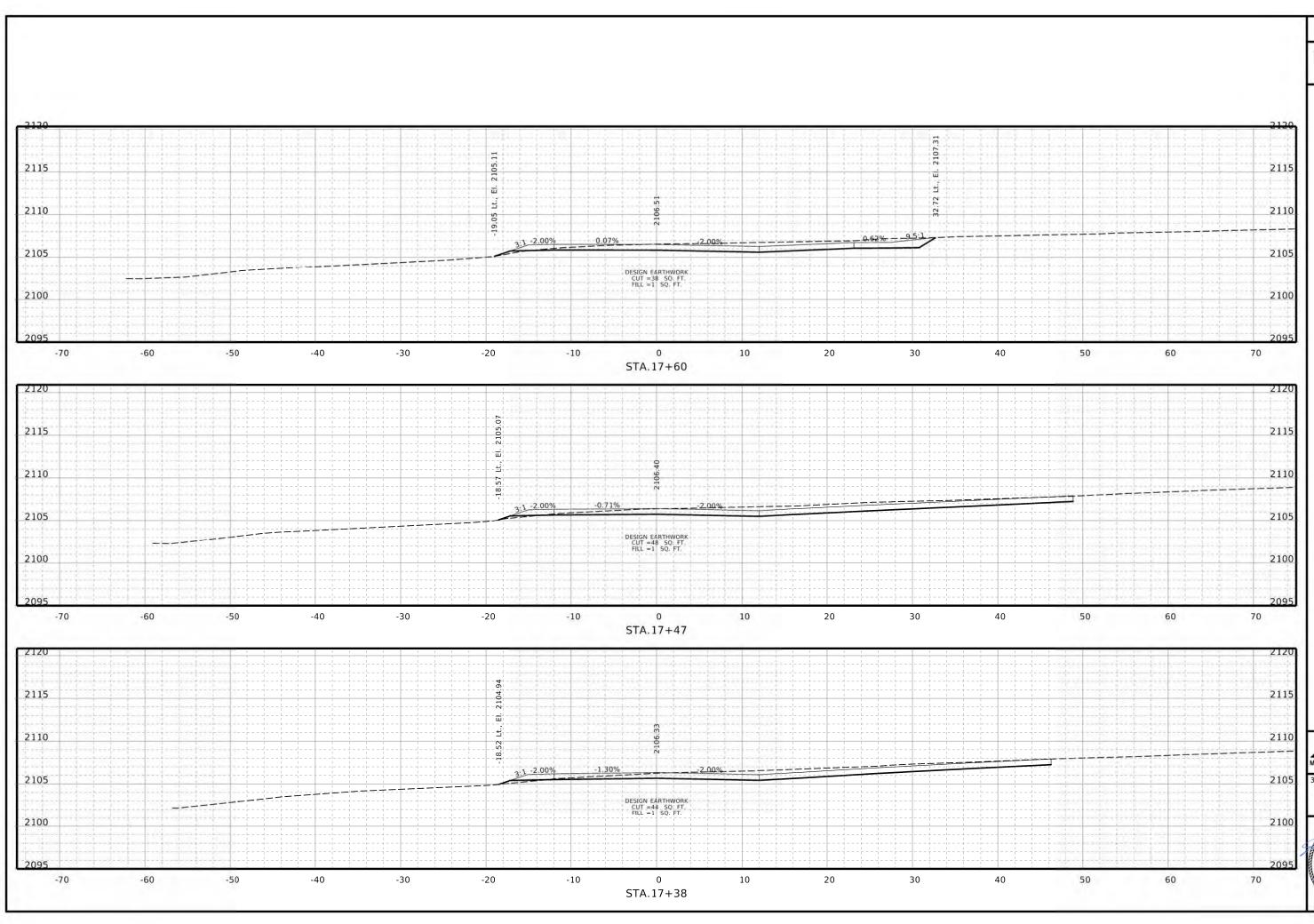
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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-R13W

CROSS SECTIONS

MEINMIDWEST ENGINEERING, INC

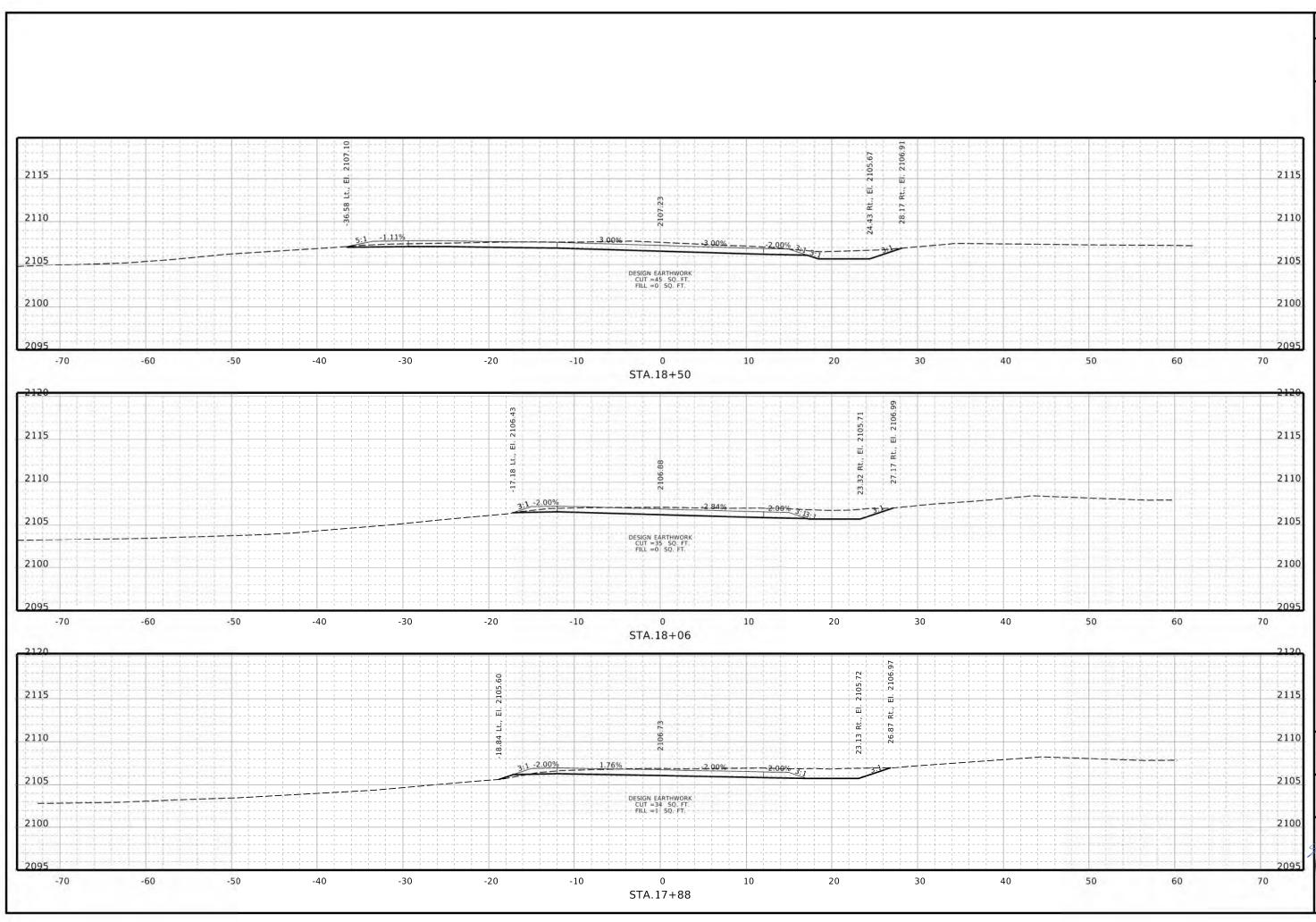




23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-RI3W
CROSS SECTIO

SECTIONS



Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT

DAVIS CREE

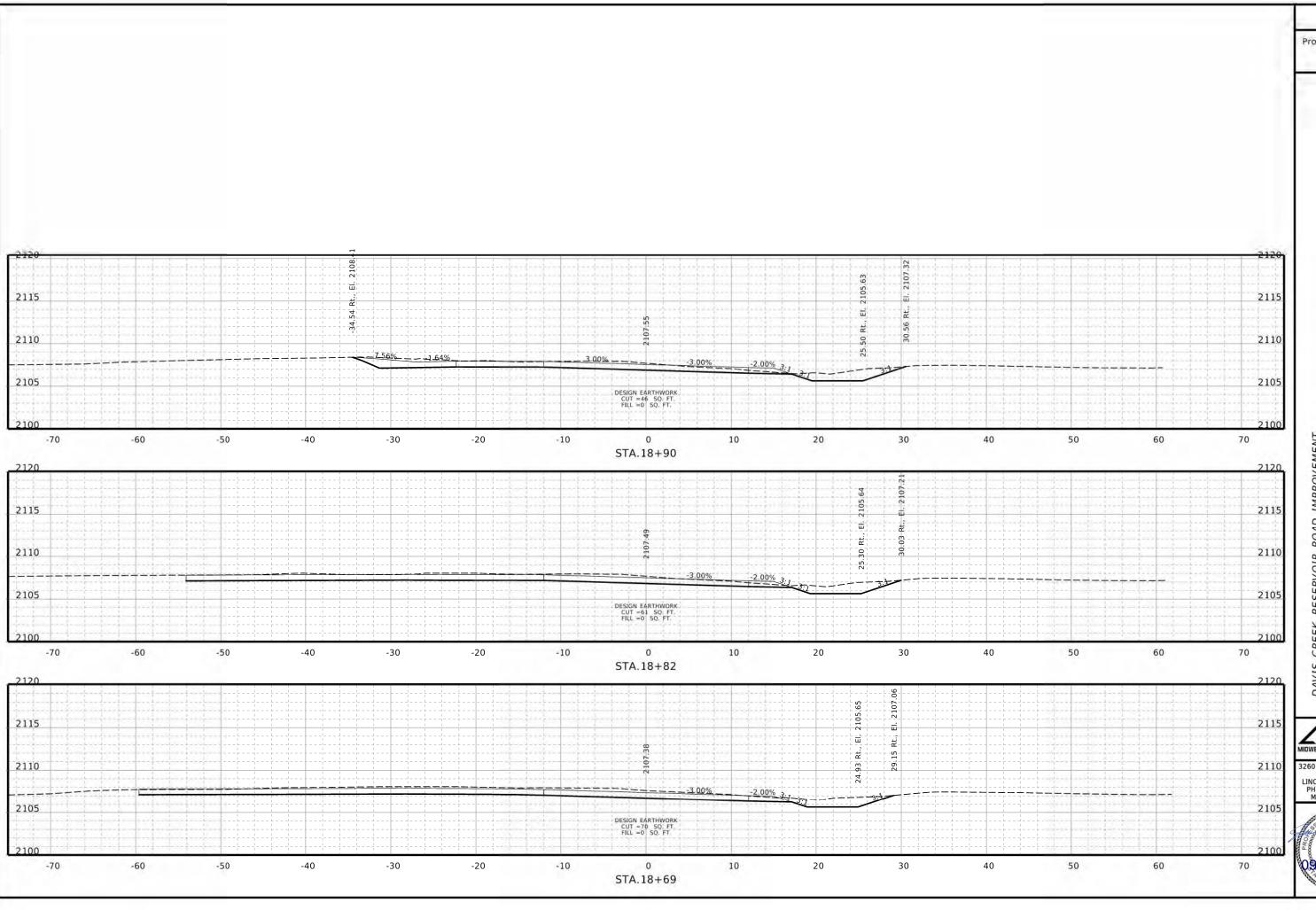
IN SEC. 25-1

CROSS

SECTIONS

MIDWEST ENGINEERING, INC 3260 FOLKWAYS BLVD SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM

JOSHUAL KEITHLEY OS/01/2023



Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-R13W

CROSS SECTIONS

MEIN MIDWEST ENGINEERING, INC



-60 -20 -70 STA.19+41 -40 -30 -20 -10 -70 STA.19+12 -70 -60 -50 -40 -30 -20 -10 STA.19+00

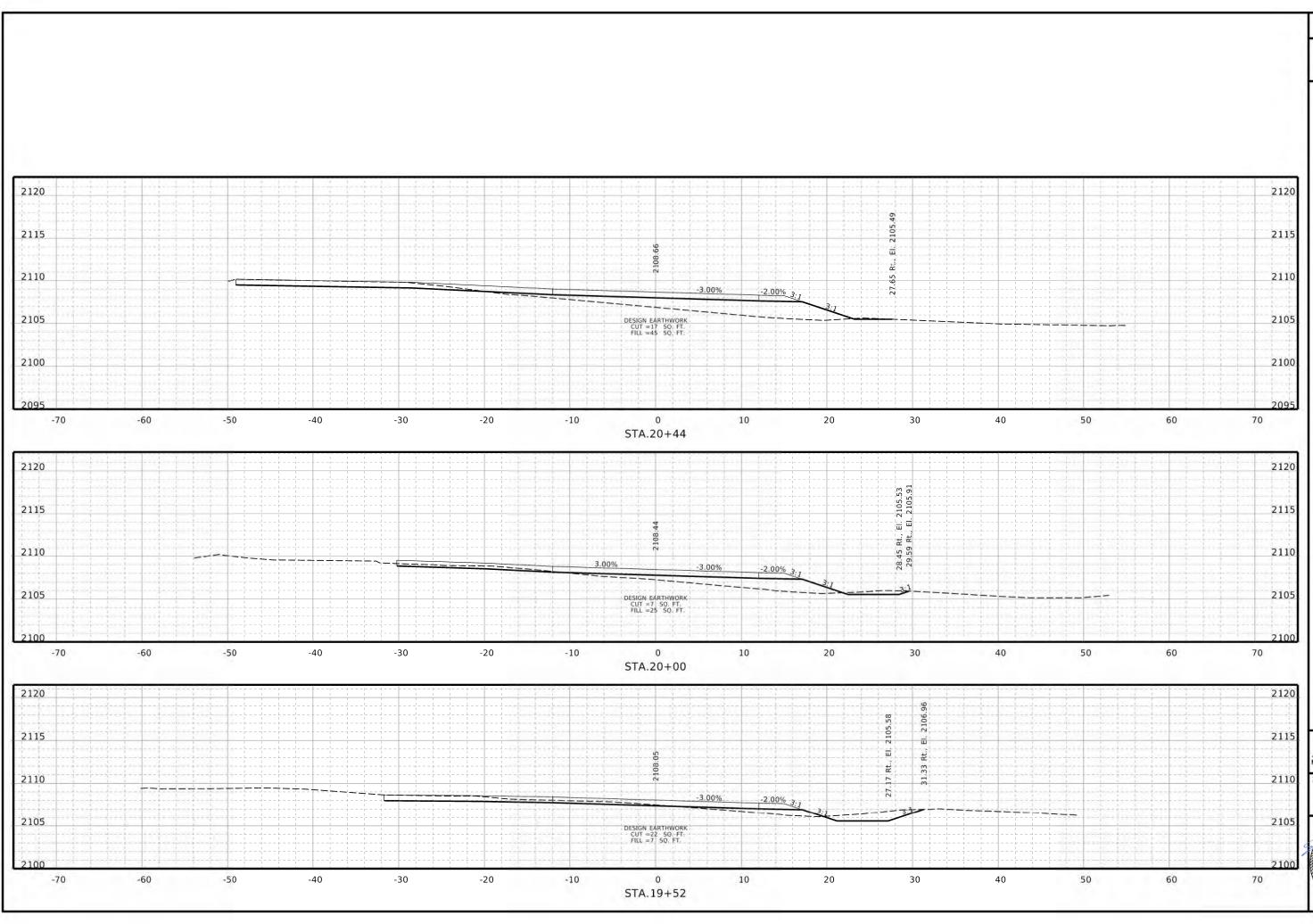
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Project Number 

> DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RIBW
> CROSS SECTION

SECTIONS





Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-R13W

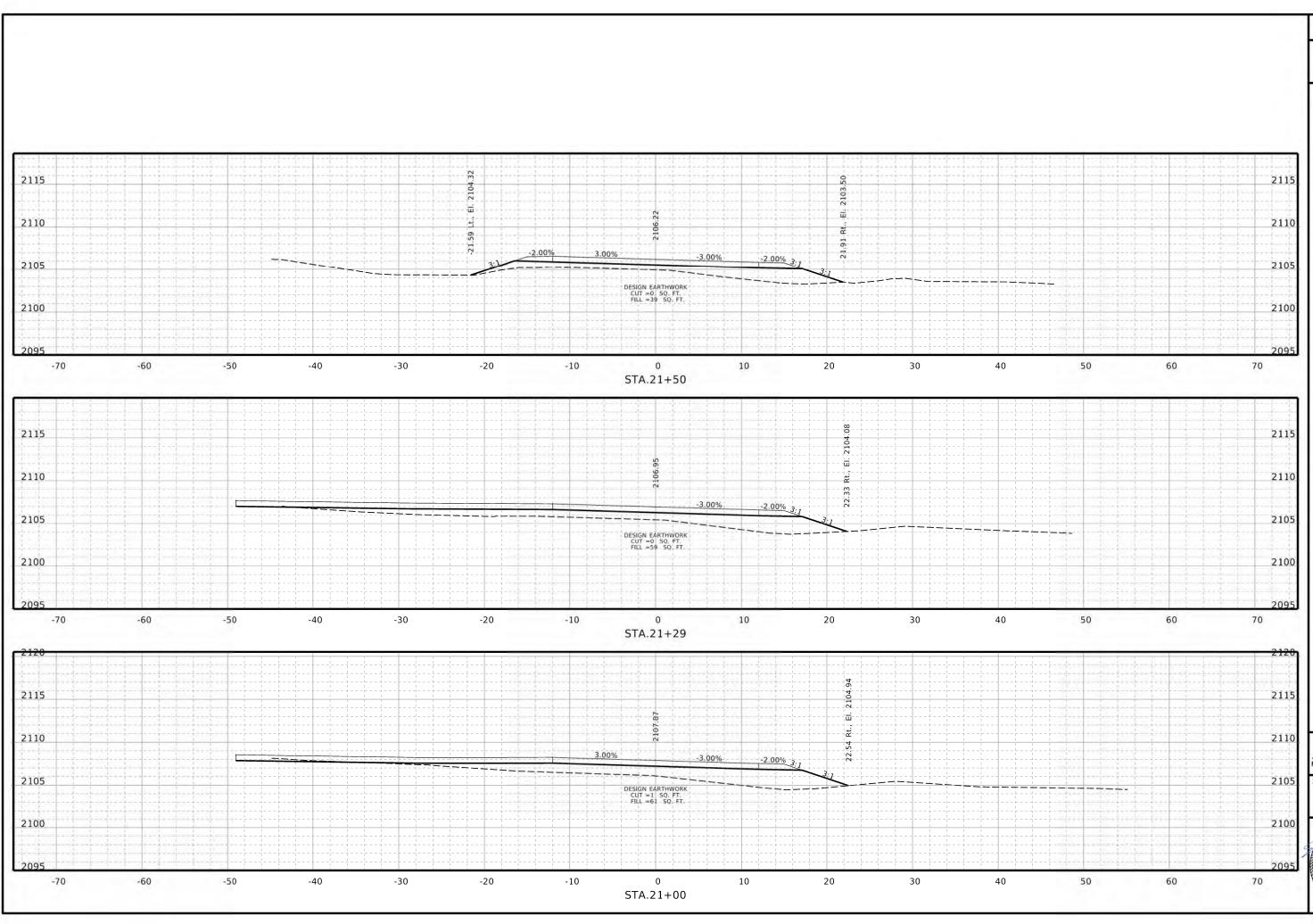
CROSS SECTIONS

MEI\
MIDWEST ENGINEERING, INC

MIDWEST ENGINEERING, INC 3260 FOLKWAYS BLVD SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM

JOSHUAL KEITHLEY

QUOTION NEBRIS



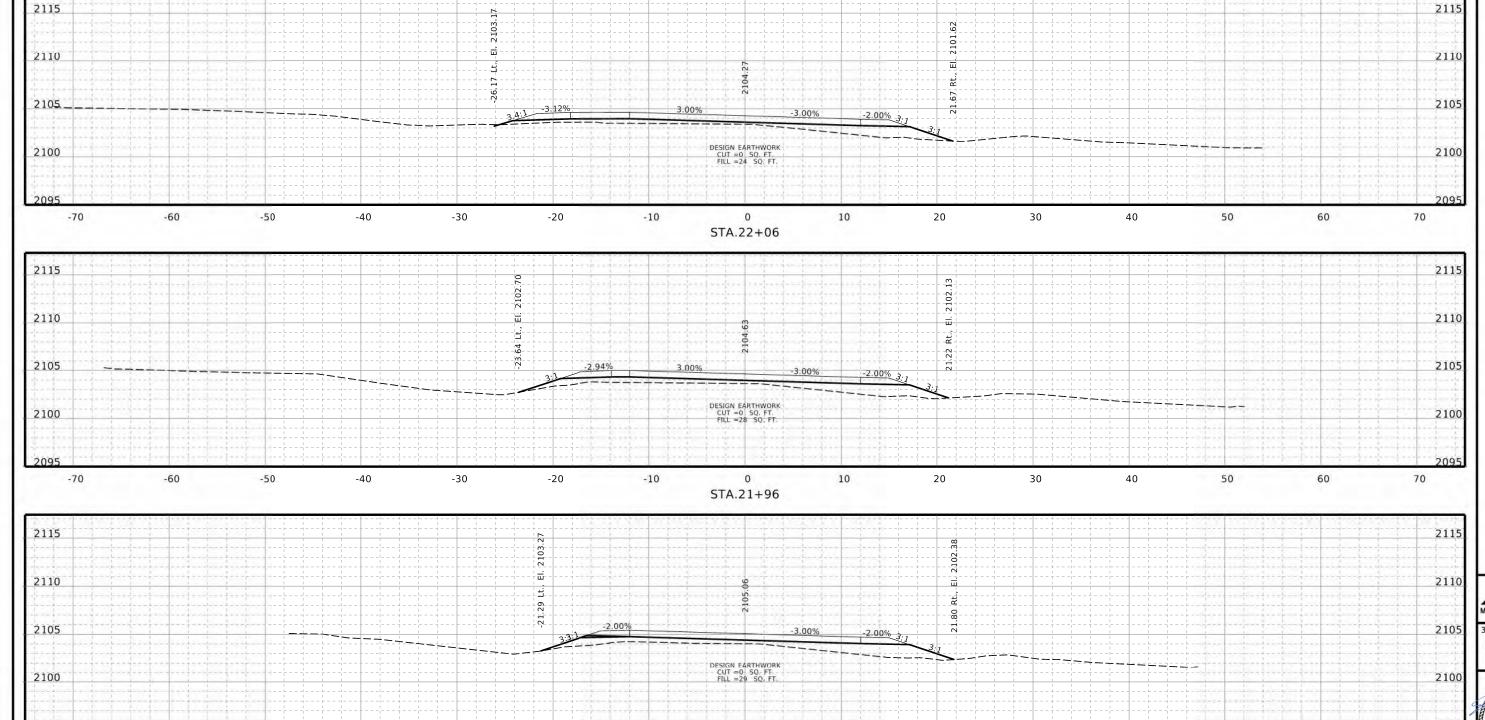
Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-T17N-R13W
CROSS SECTION

SECTIONS



23020



0

STA.21+83

10

20

-60

-40

-30

-20

-10

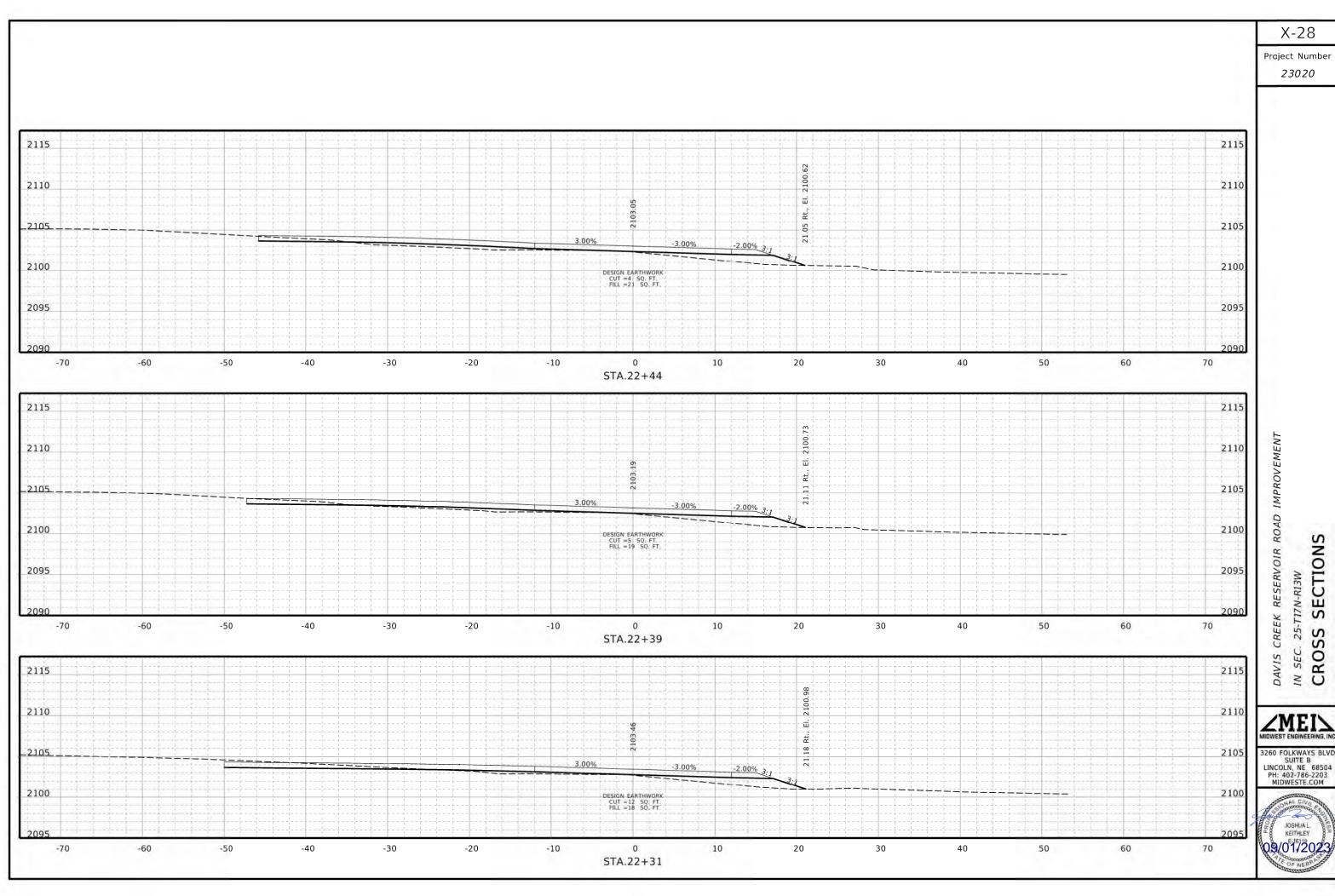
DAVIS CREEK RESERVOIR ROAD IMPROVEMENT SECTIONS IN SEC. 25-T17N-R13W
CROSS SECTIO

3260 FOLKWAYS BLVE SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM



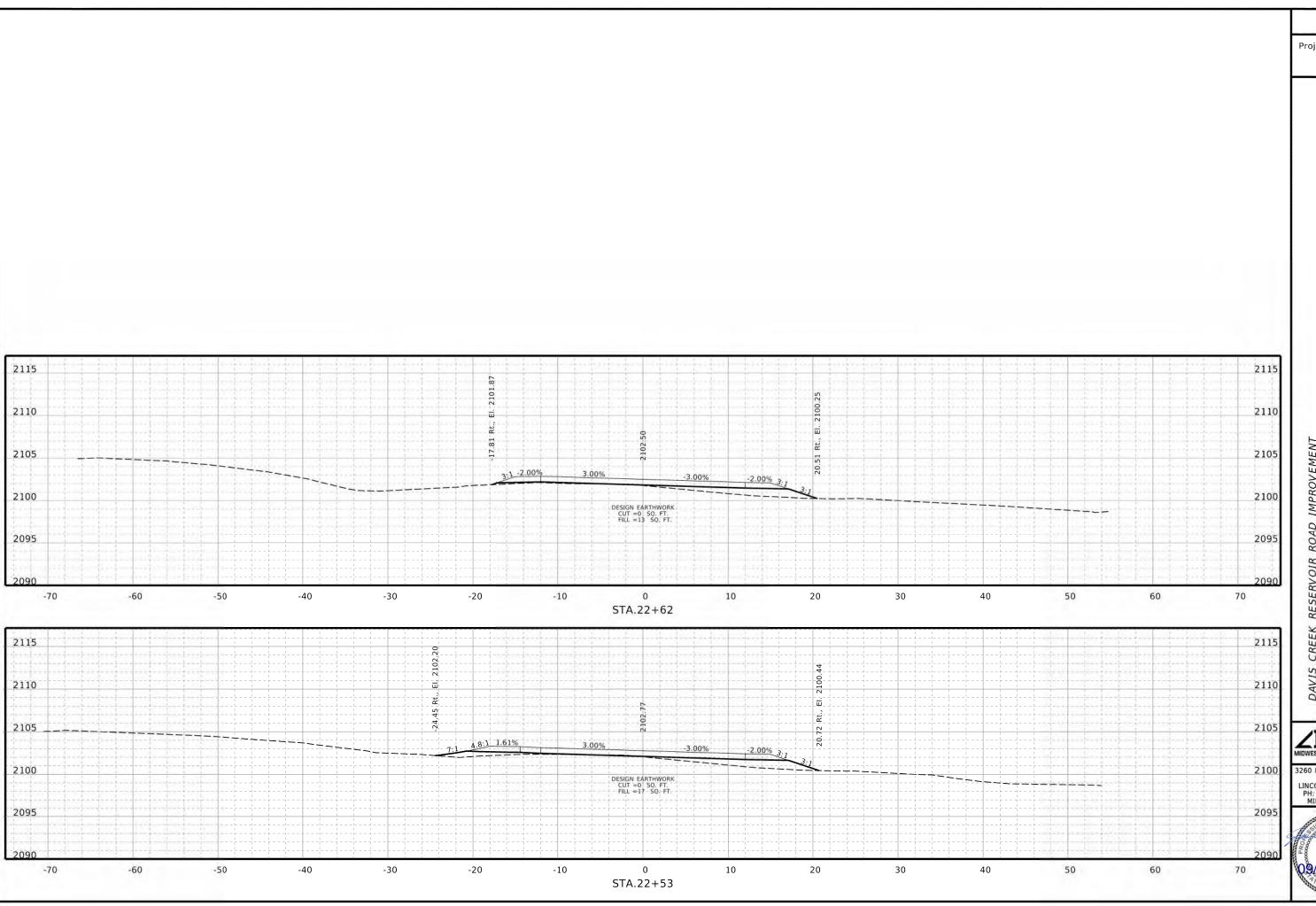
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70



Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W
CROSS SECTIONS



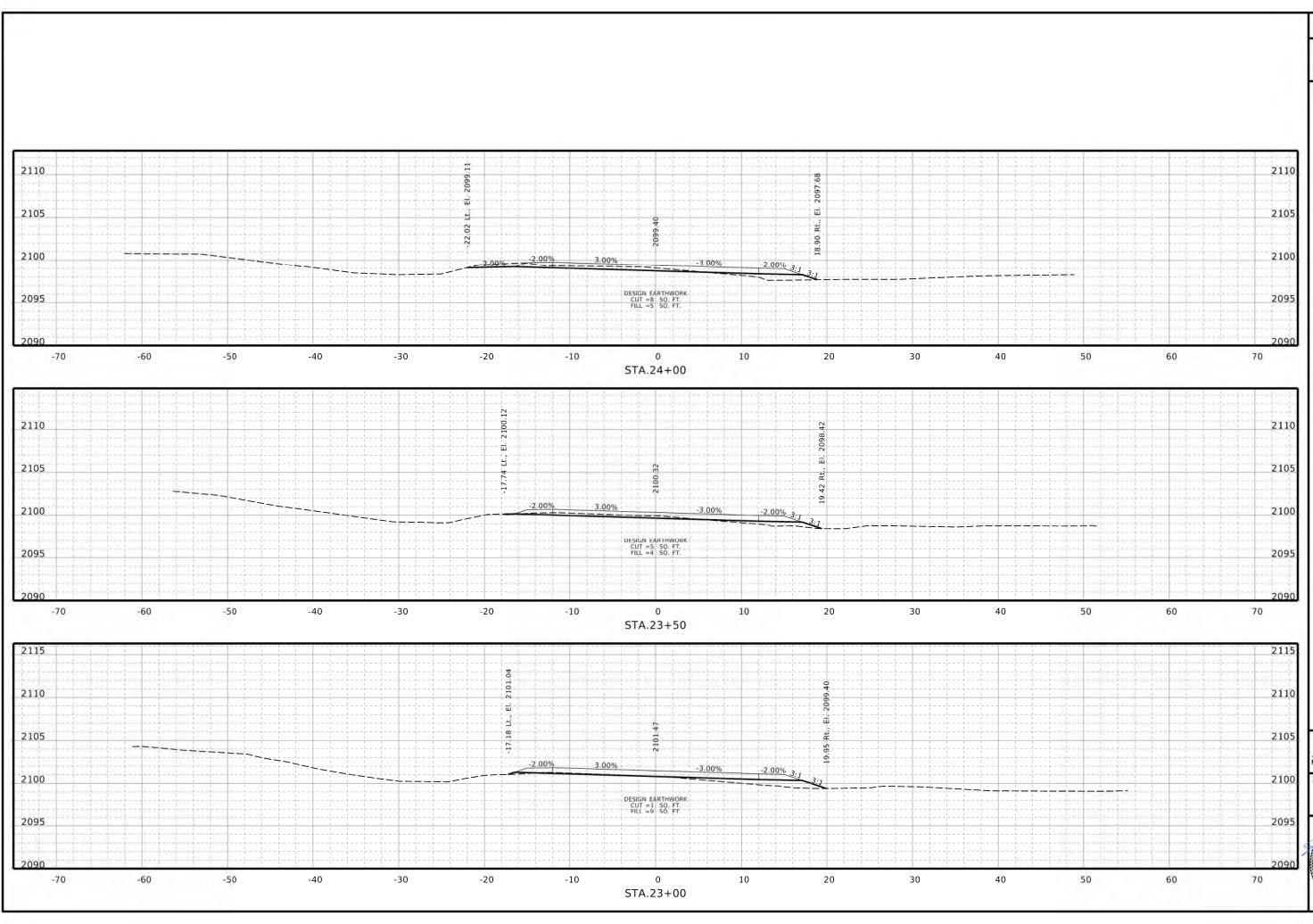
Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W

CROSS SECTIONS

MEI\
MIDWEST ENGINEERING, INC





23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-7

SECTIONS

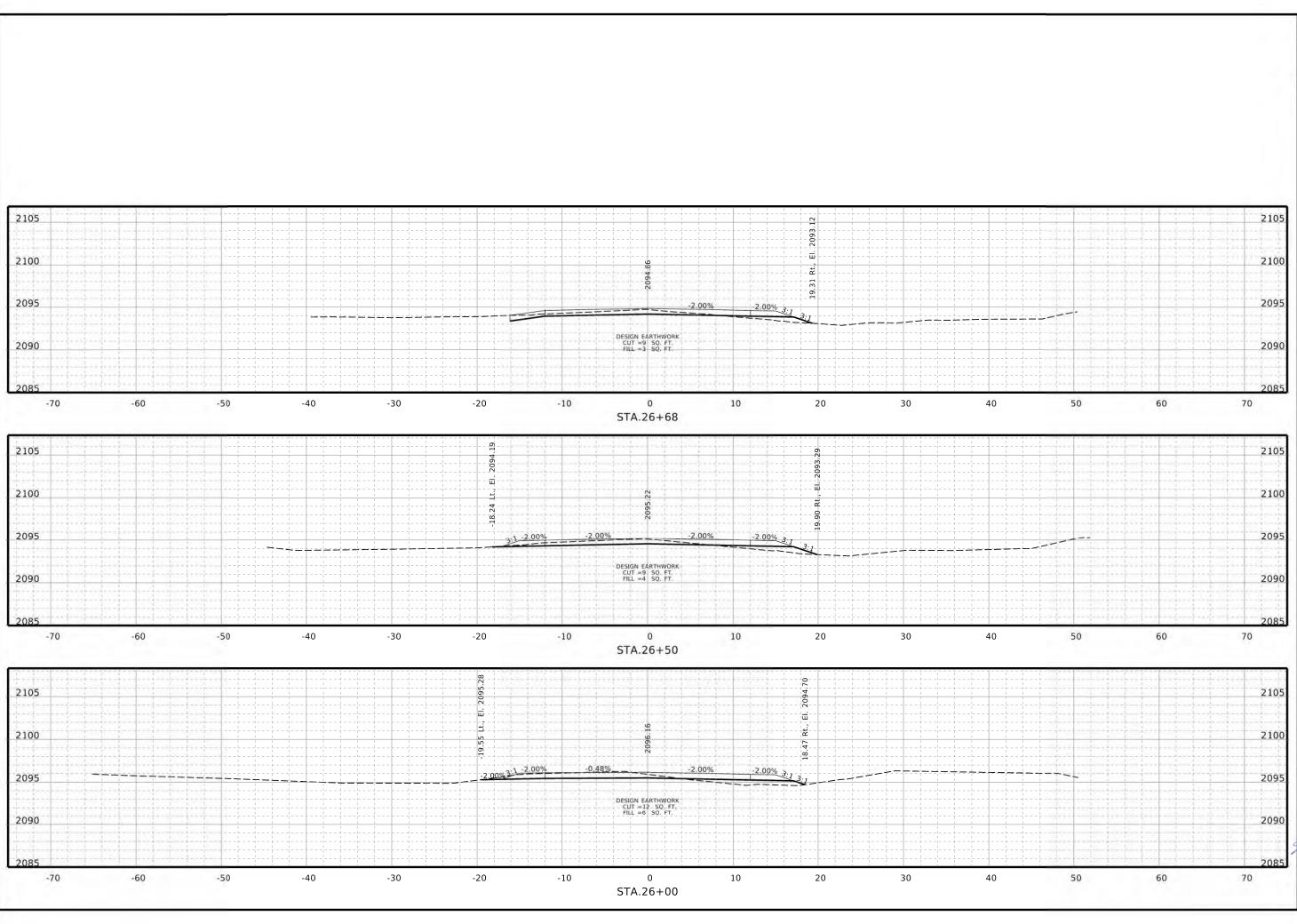


Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W
CROSS SECTIC

SECTIONS



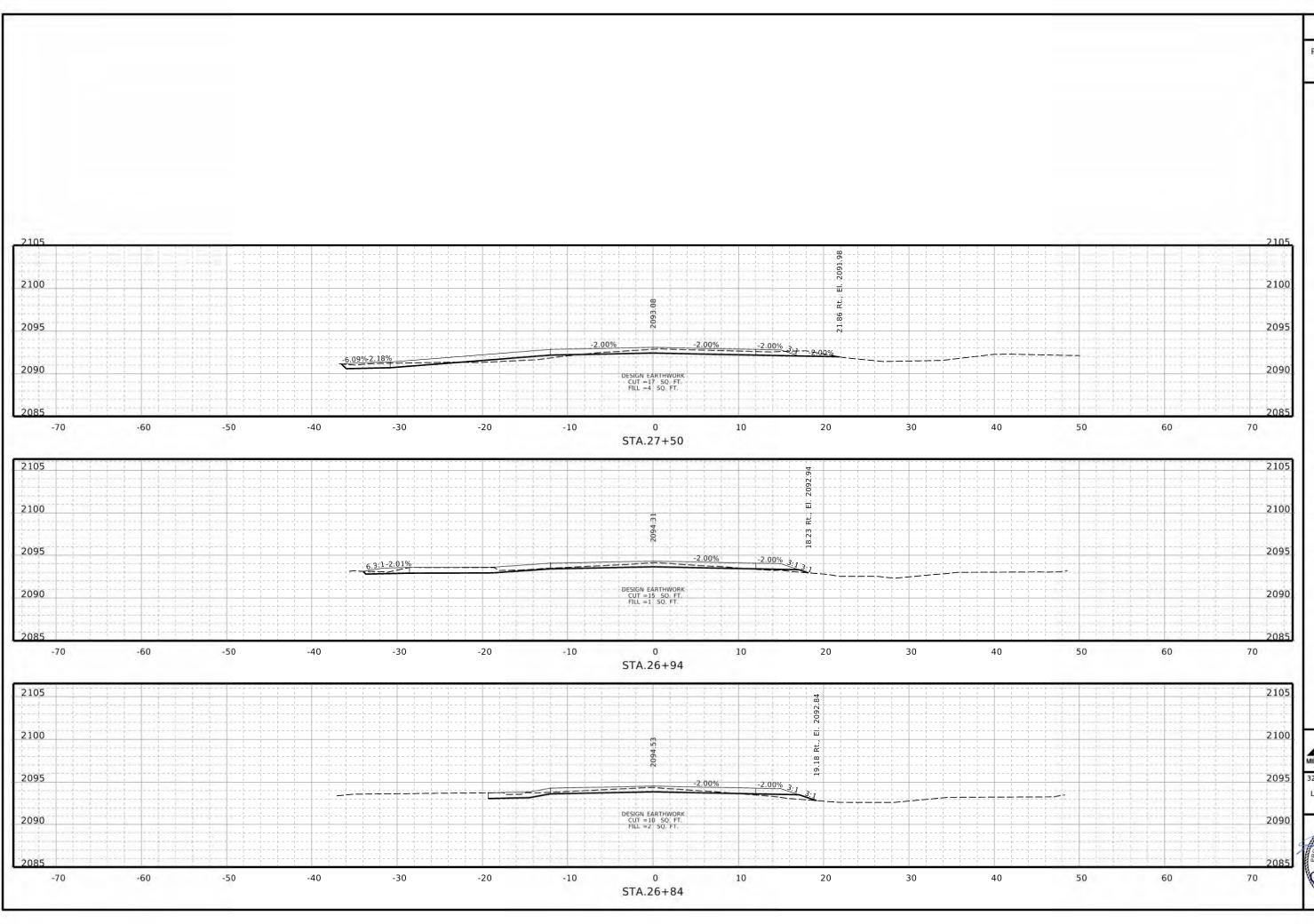


Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-RI3W
CROSS SECTIC

SECTIONS





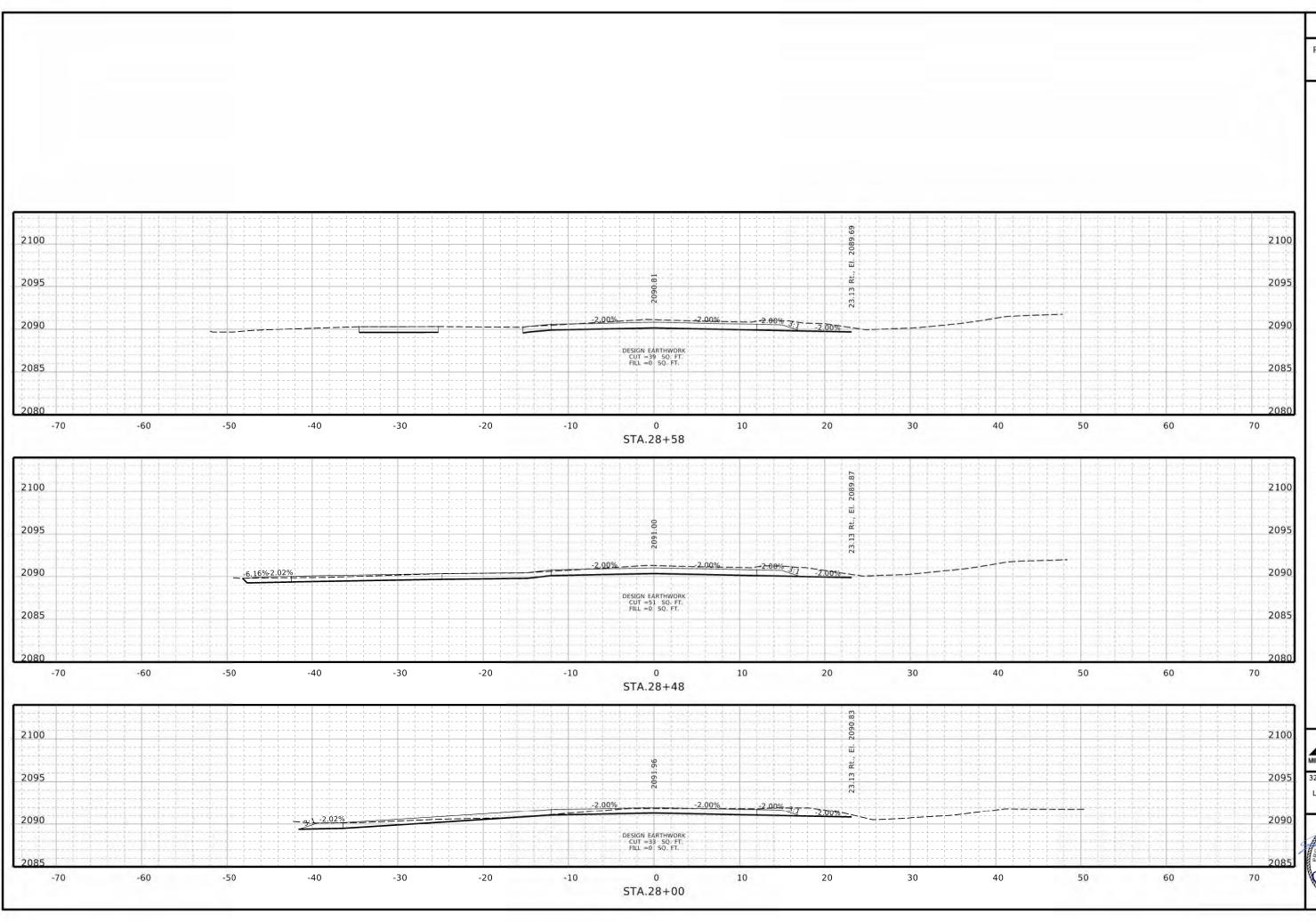
Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W

CROSS SECTIONS

MEI MIDWEST ENGINEERING, INC



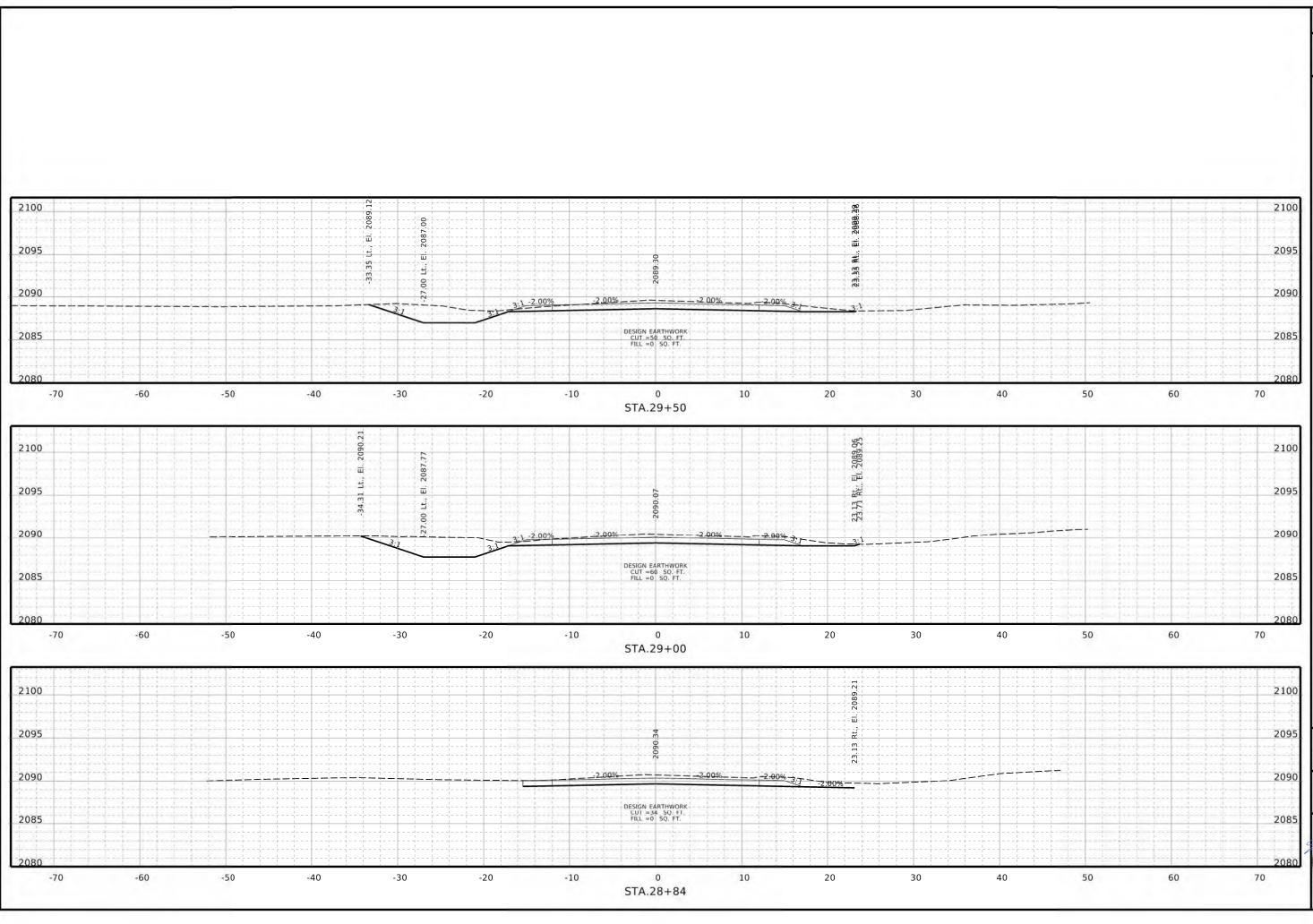


Project Number 23020

> DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RIBW
> CROSS SECTIO

SECTIONS



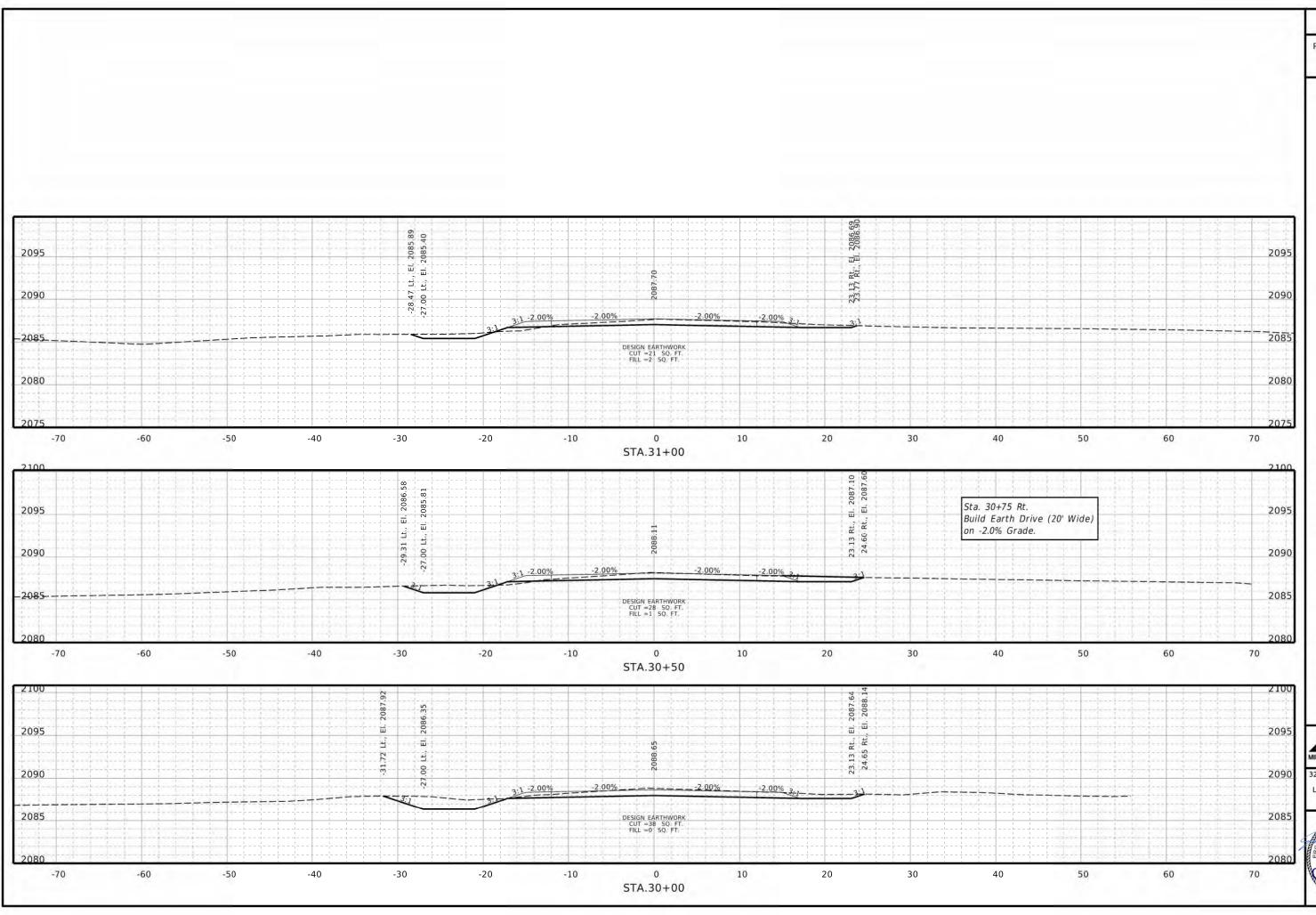


Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-RI3W
CROSS SECTION

SECTIONS



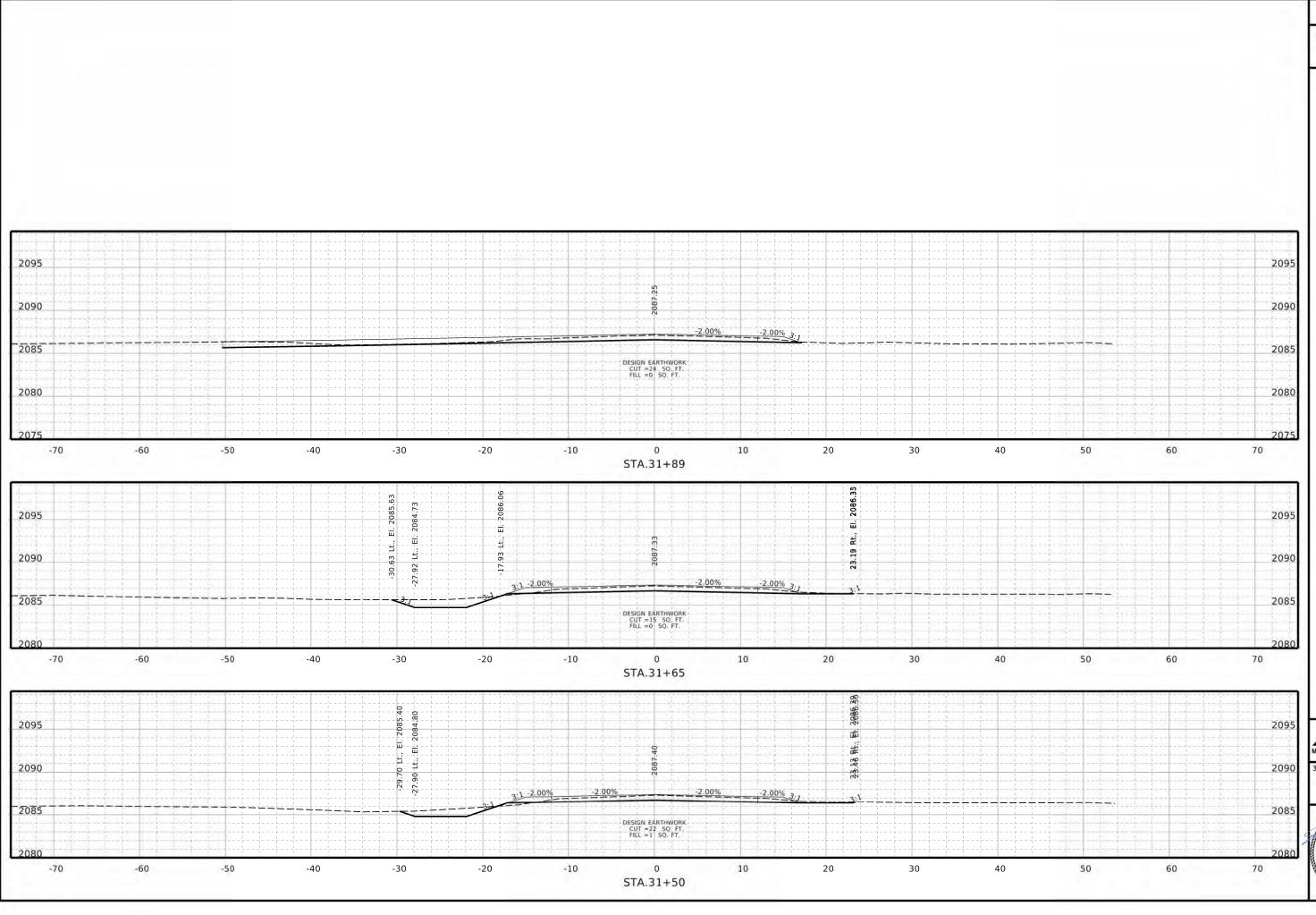


Project Number 23020

> DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-T17N-R13W
> CROSS SECTIC

SECTIONS



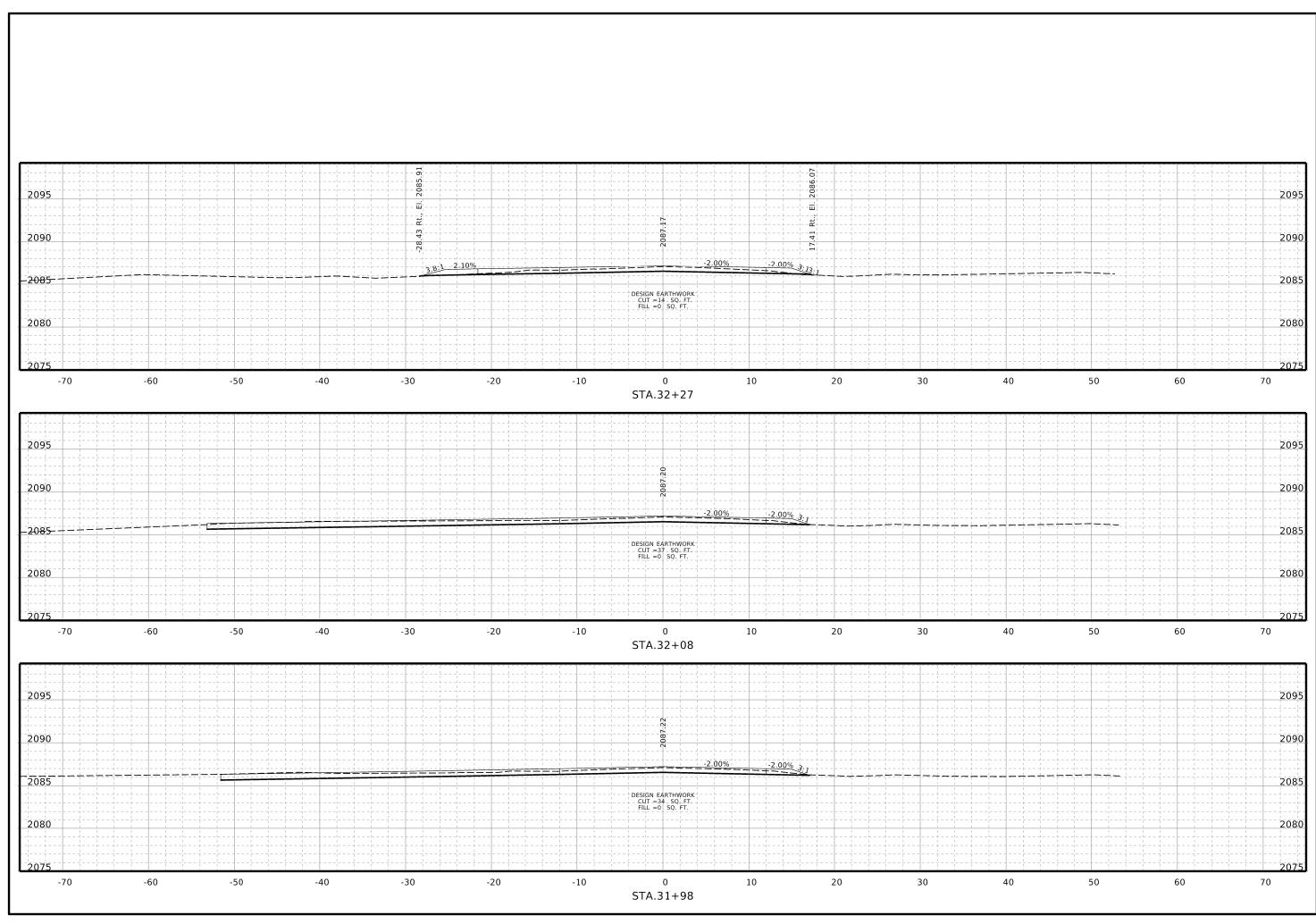


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> CROSS SECTIC

SECTIONS



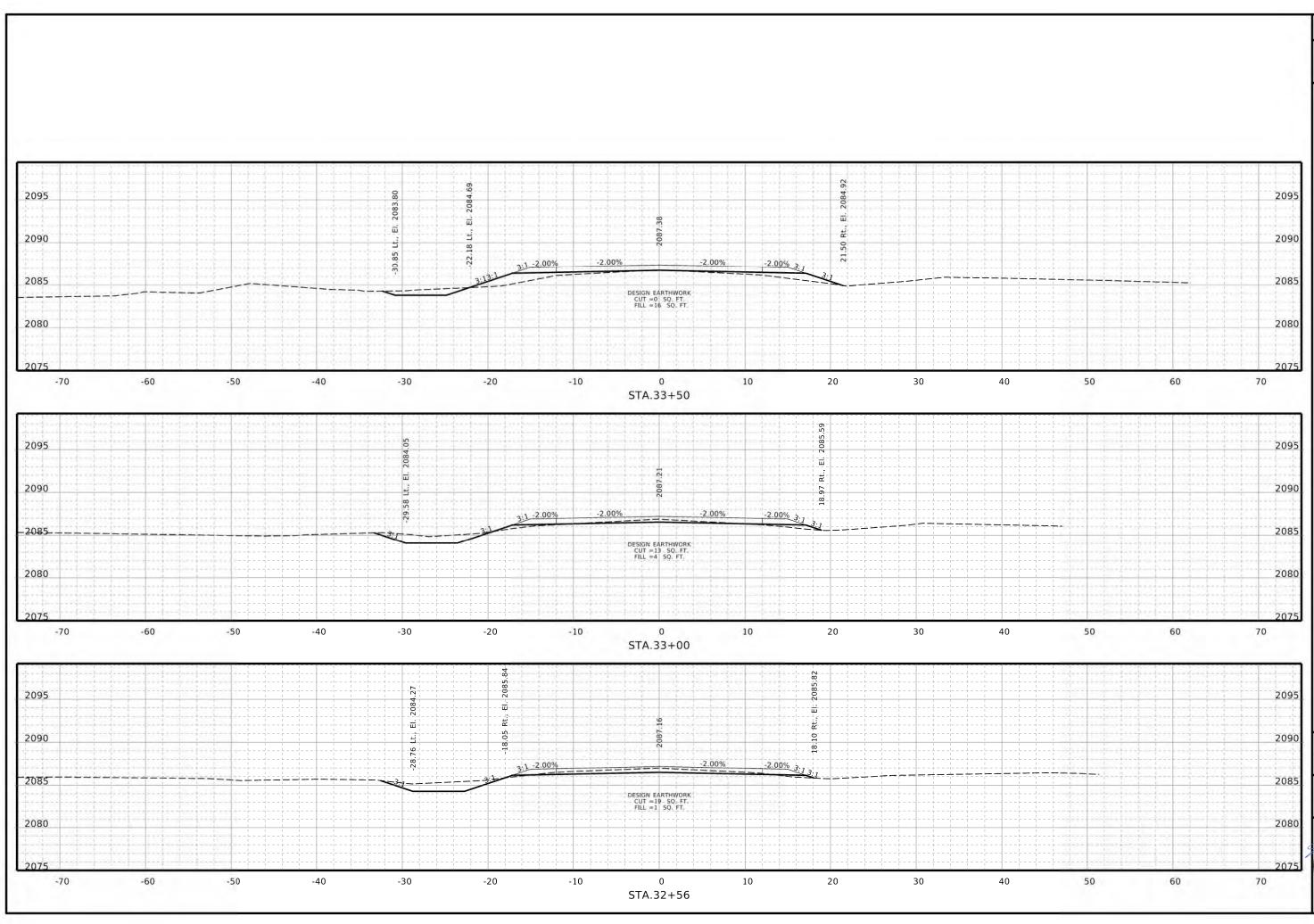


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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-RI3W
CROSS SECTIC

SECTIONS

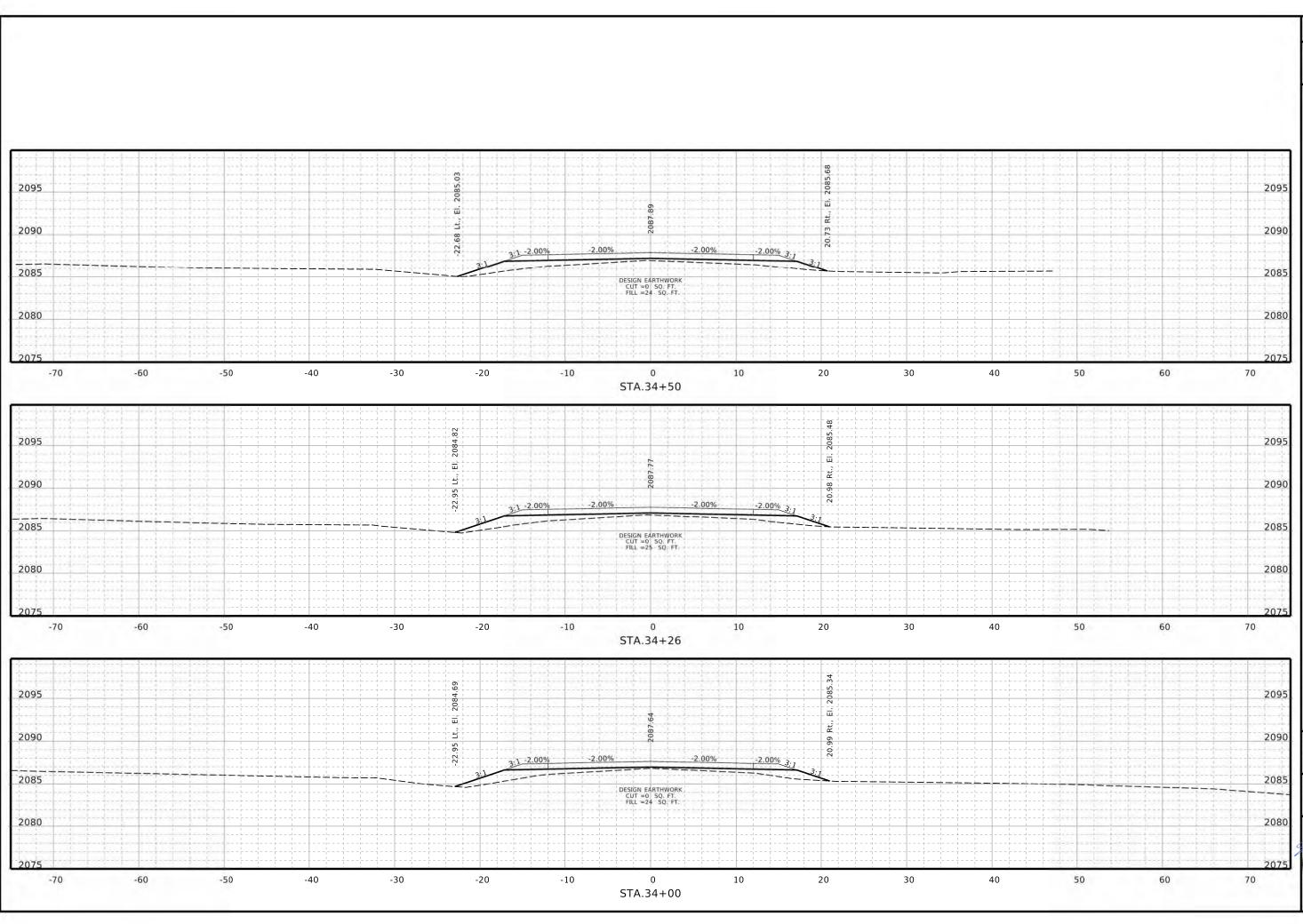




Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W
CROSS SECTION

SECTIONS

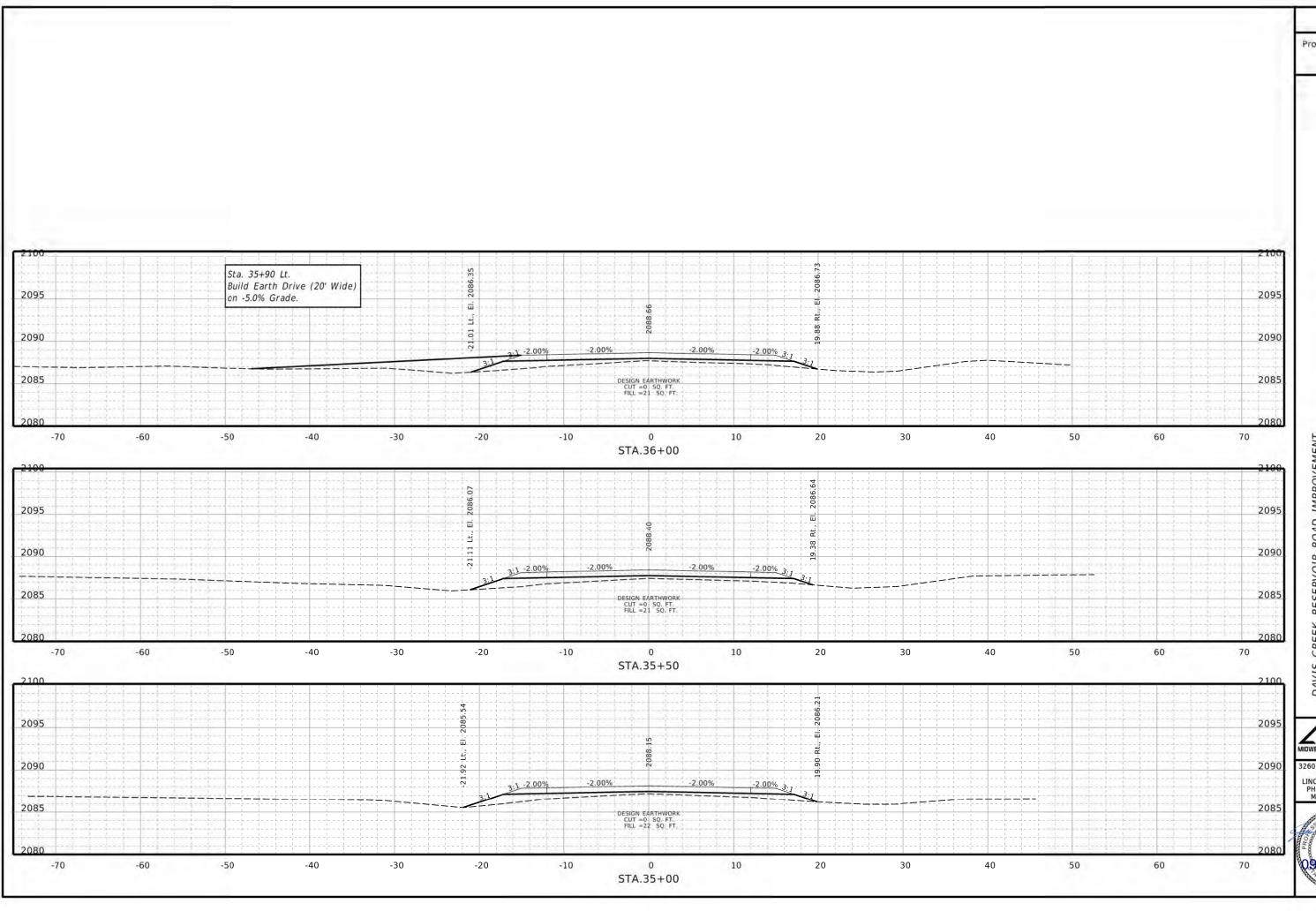


23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-7

SECTIONS



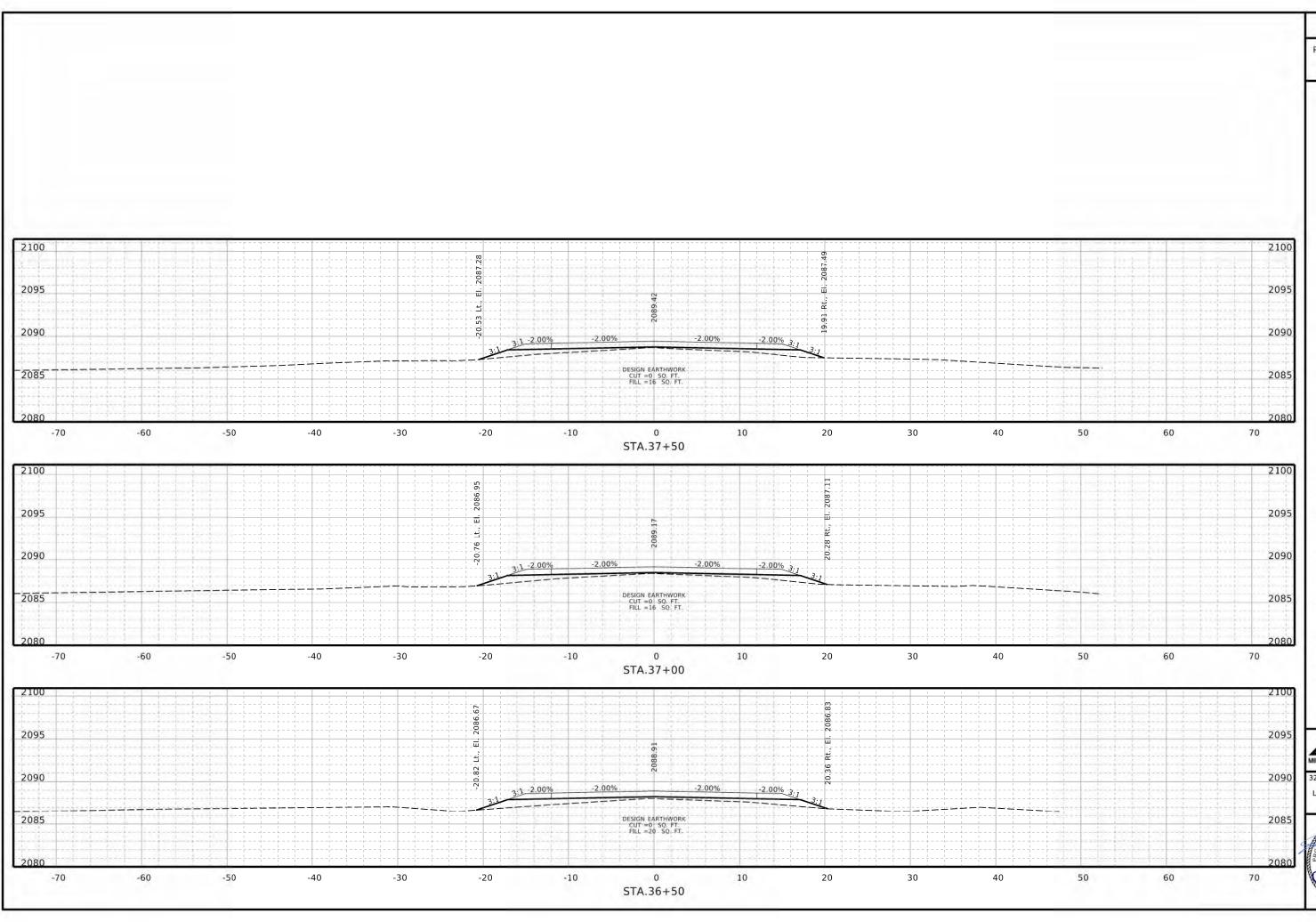


Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RIBW
CROSS SECTIO

SECTIONS



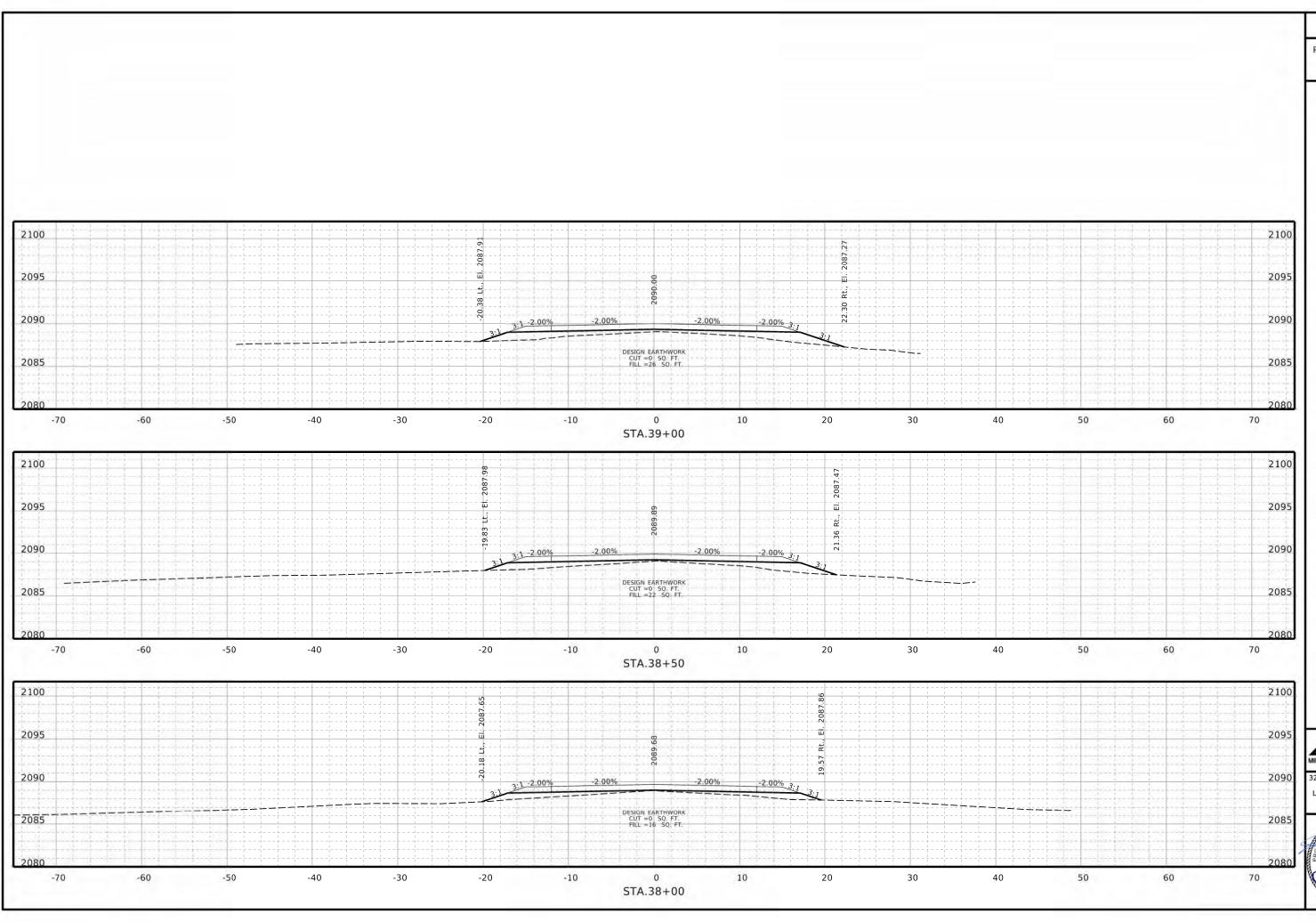


Project Number 23020

> DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W
> CROSS SECTIC

SECTIONS





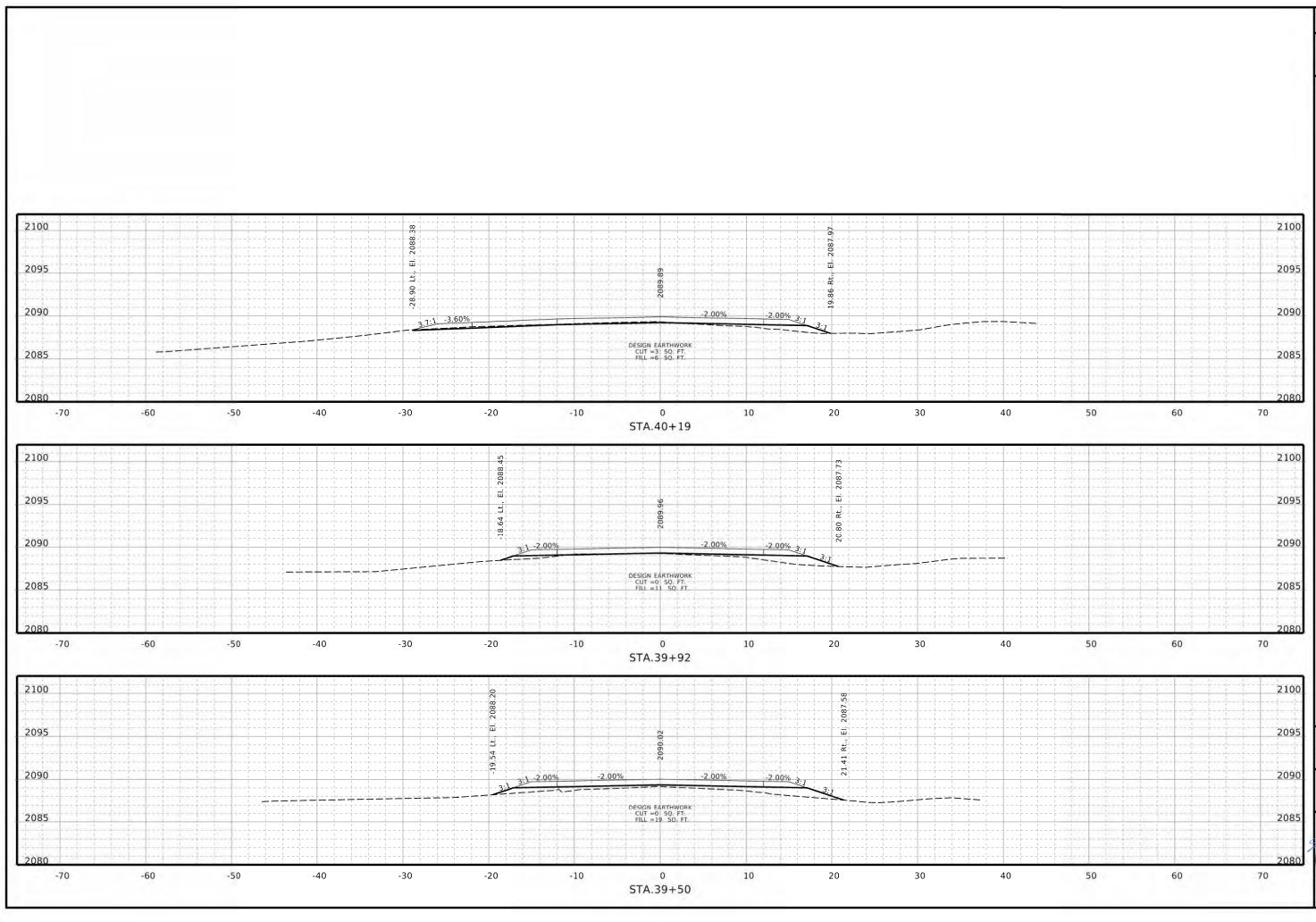
Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W

CROSS SECTIONS

MEI
MIDWEST ENGINEERING, INC
3260 FOLKWAYS BLVD





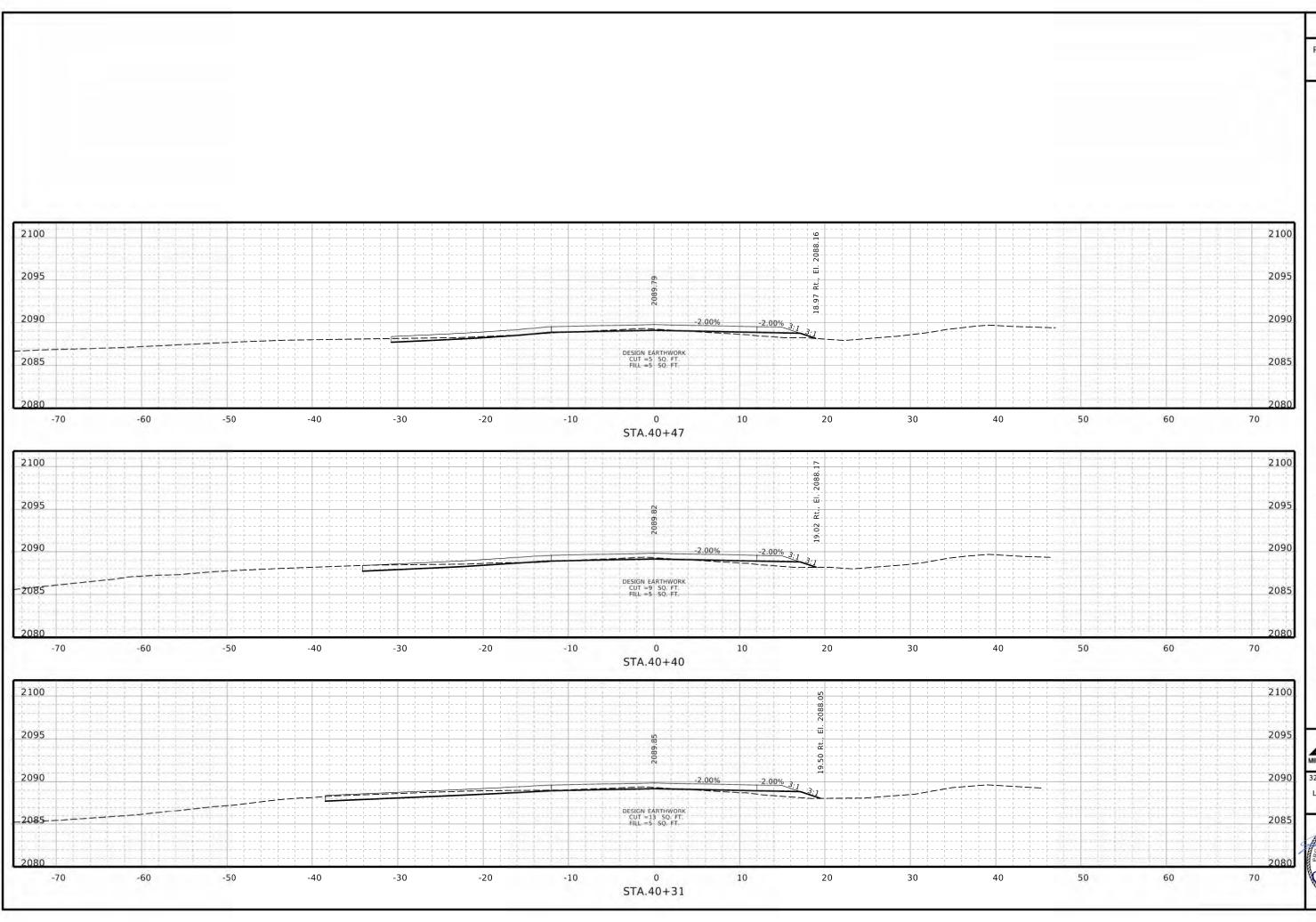
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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W

CROSS SECTIONS

MEIN MIDWEST ENGINEERING, INC





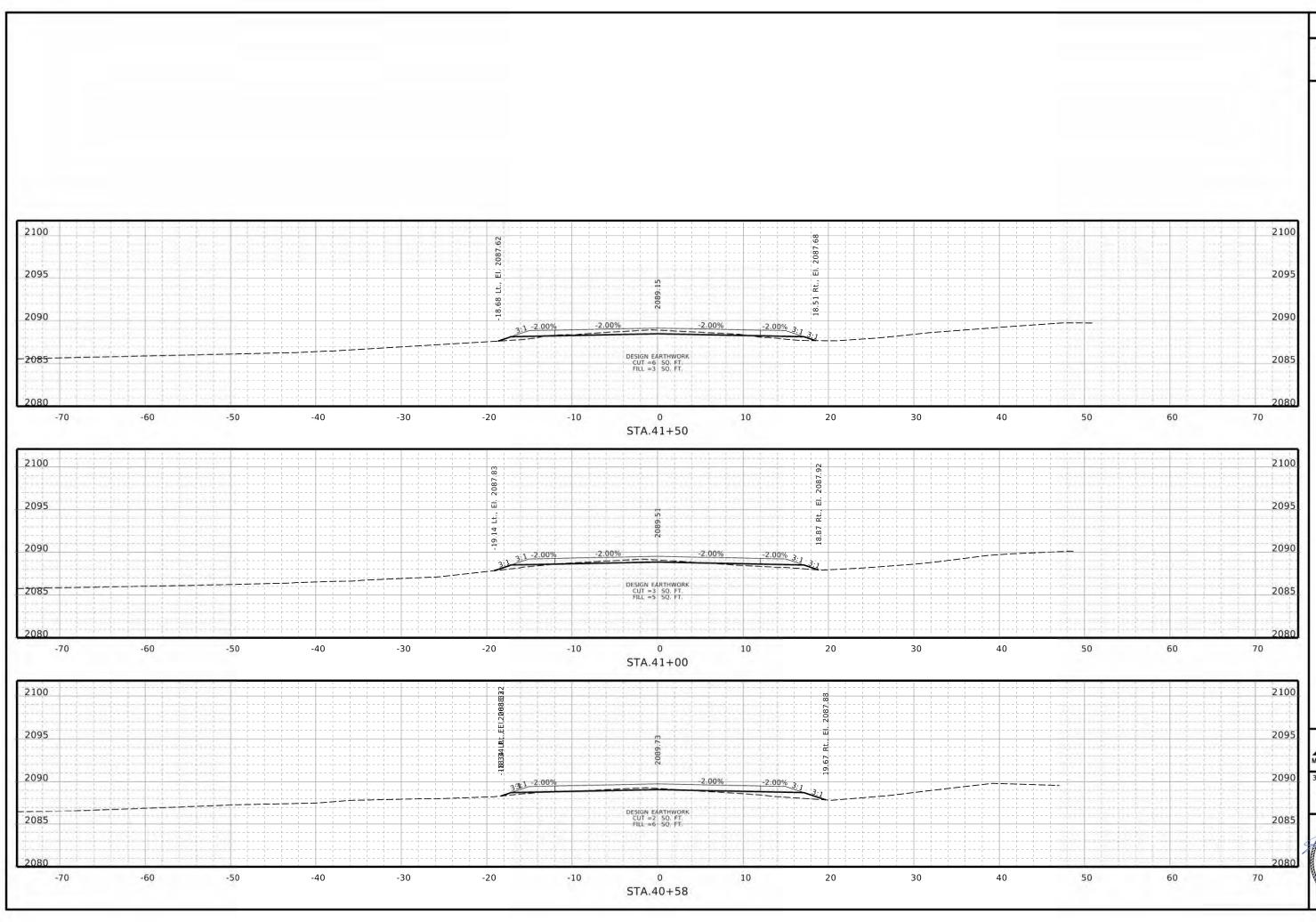
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DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W

CROSS SECTIONS

MEIN MIDWEST ENGINEERING, INC





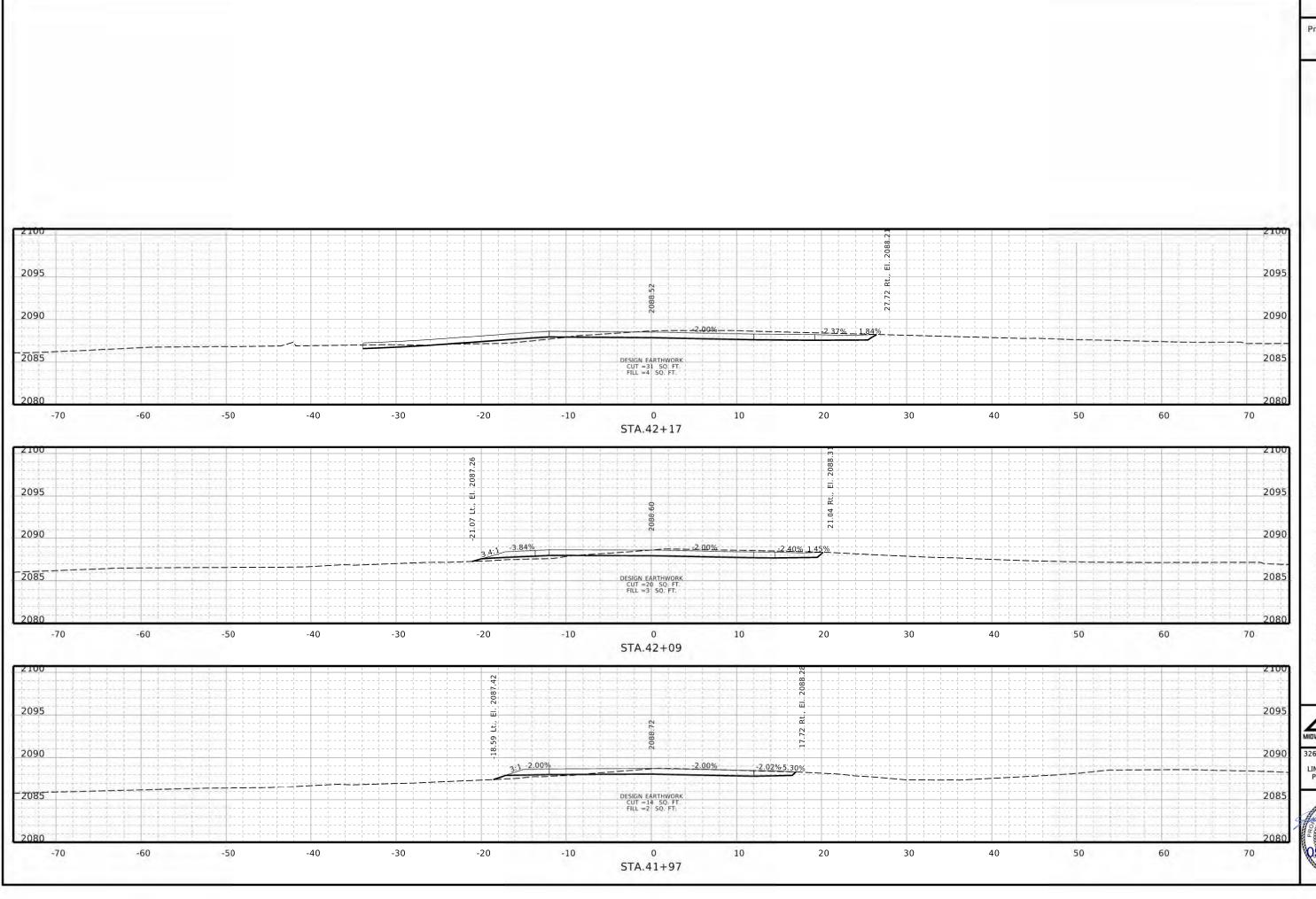
Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W

CROSS SECTIONS

MEIX
MIDWEST ENGINEERING, INC





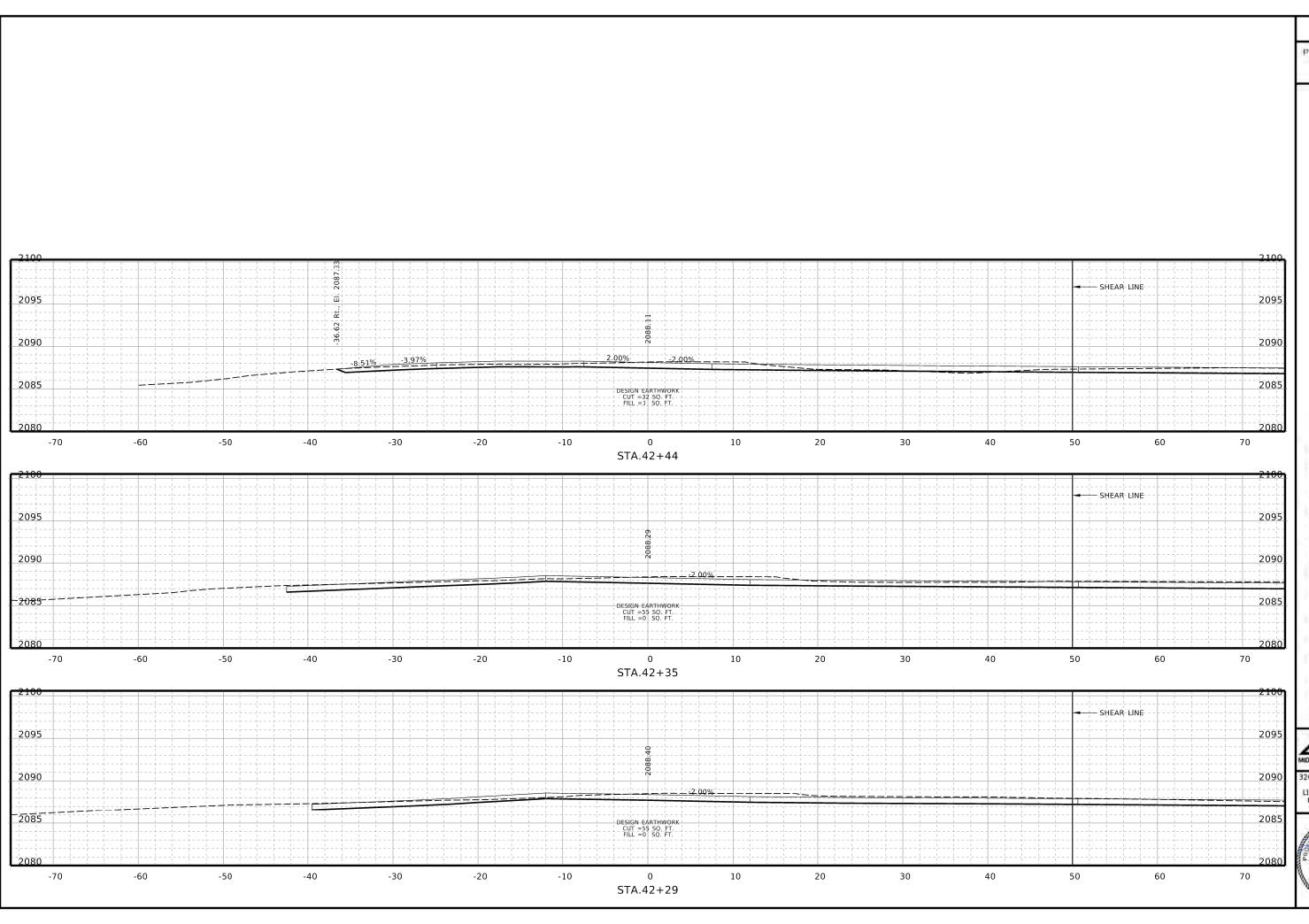
Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-T17N-R13W

CROSS SECTIONS

MEIN MIDWEST ENGINEERING, INC



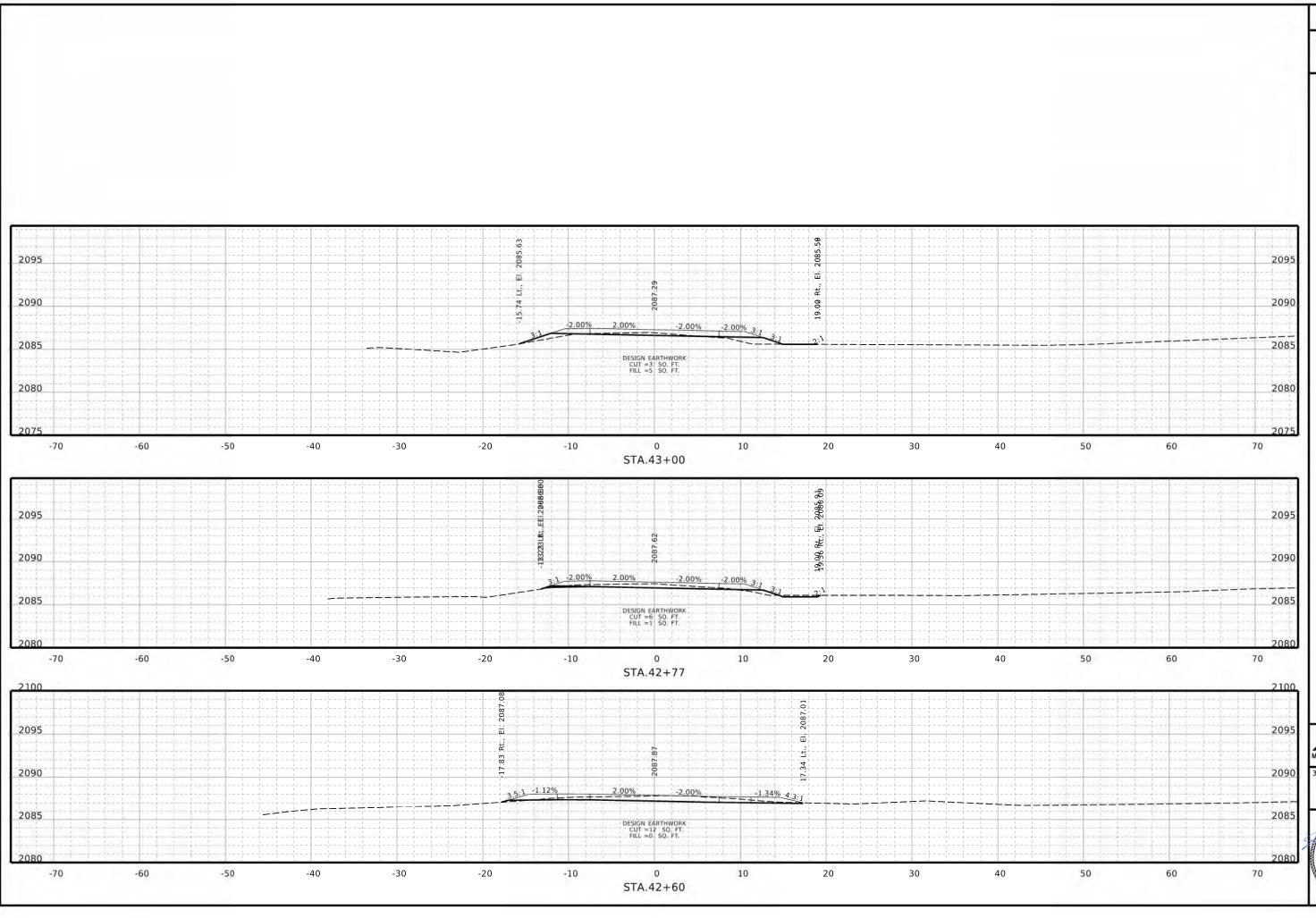


Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W
CROSS SECTIONS

MEIX
MIDWEST ENGINEERING, INC





Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W

CROSS SECTIONS

MEI MIDWEST ENGINEERING, INC



Project Number 2095 2095 2090 2090 DAVIS CREEK RESERVOIR ROAD IMPROVEMENT 2085 2080 2080 2075 2075 -50 -40 -30 -20 10 50 60 70 -70 -60 -10 0 20 STA.44+00 2095 2095 2090 2090 2085 2080 2080 -60 -40 -30 -20 -10 0 10 20 50 60 70 STA.43+50

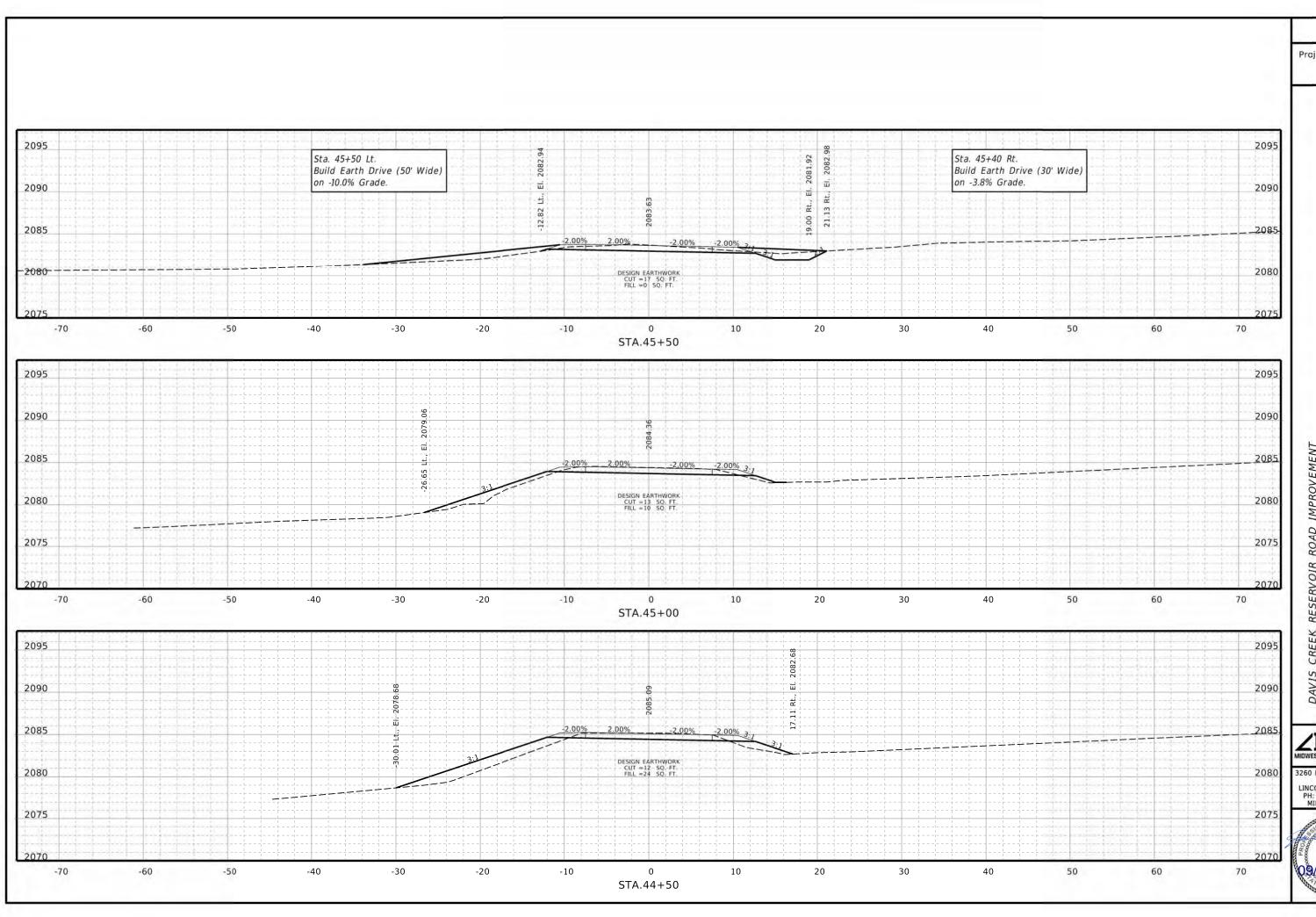
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23020

IN SEC. 25-TITN-RI3W
CROSS SECTIC

SECTIONS

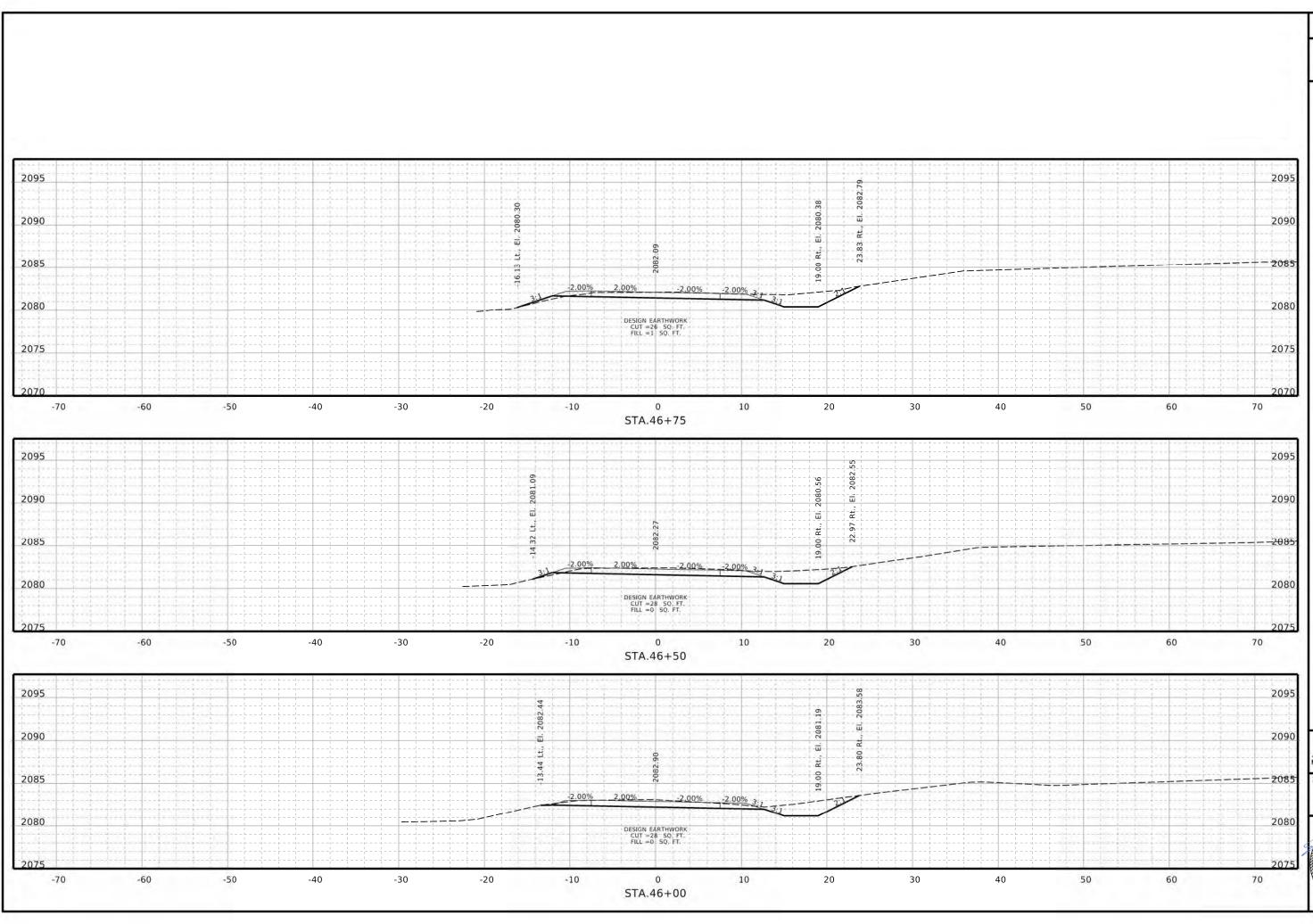




Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-7

SECTIONS



Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W
CROSS SECTIC

SECTIONS



2095 2095 2090 2090 2085 2085 2080 2080 2075 2075 -40 -30 -20 20 30 40 50 60 70 -60 -10 10 -70 0 STA.47+15 2095 2090 2090 2085 2080 2080 2075 2075 -70 -60 -40 -30 -20 -10 0 10 20 50 60 70 STA.47+00

X-53

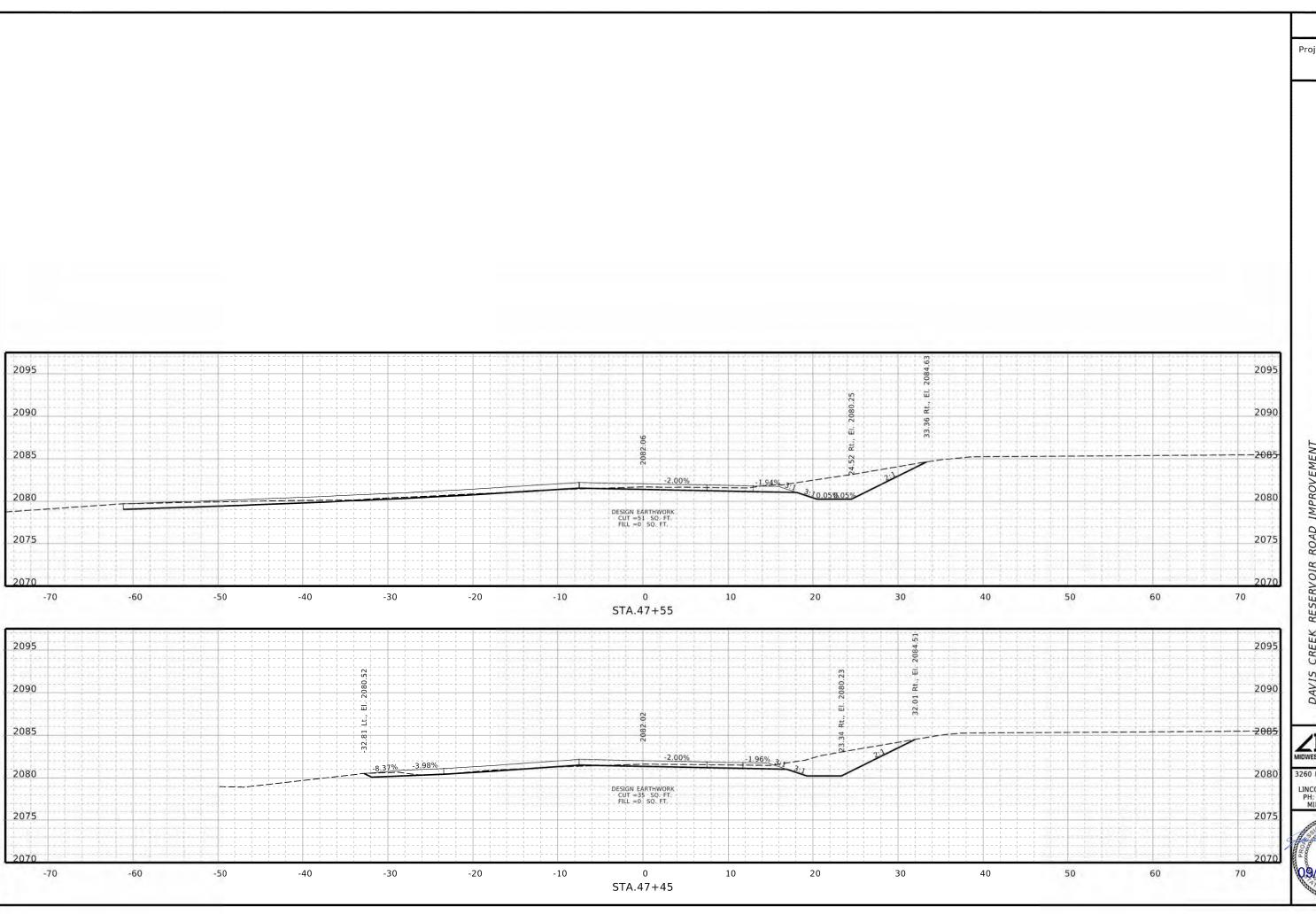
Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W

CROSS SECTIONS





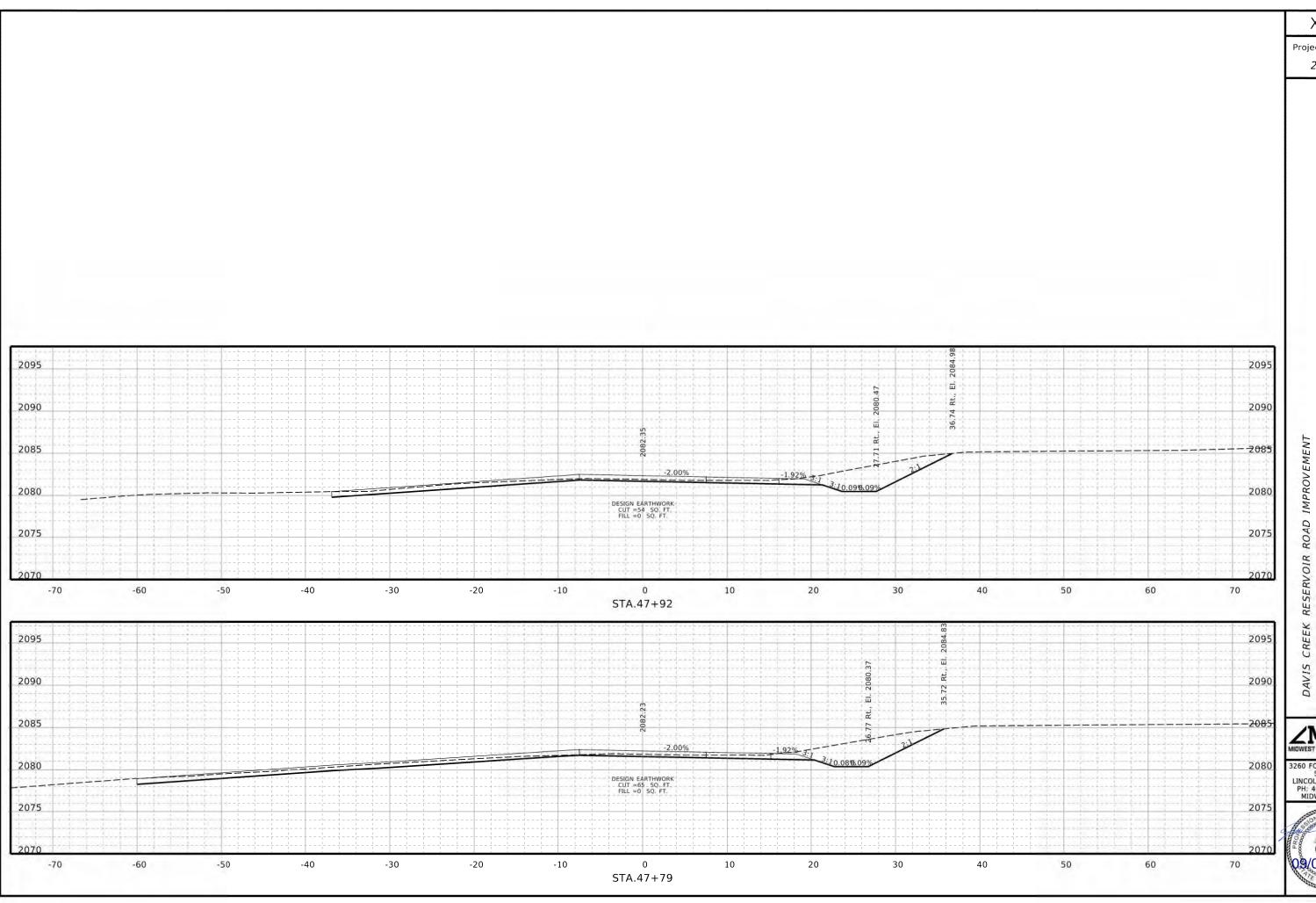


Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-R13W
CROSS SECTIONS

MEIS MIDWEST ENGINEERING, INC



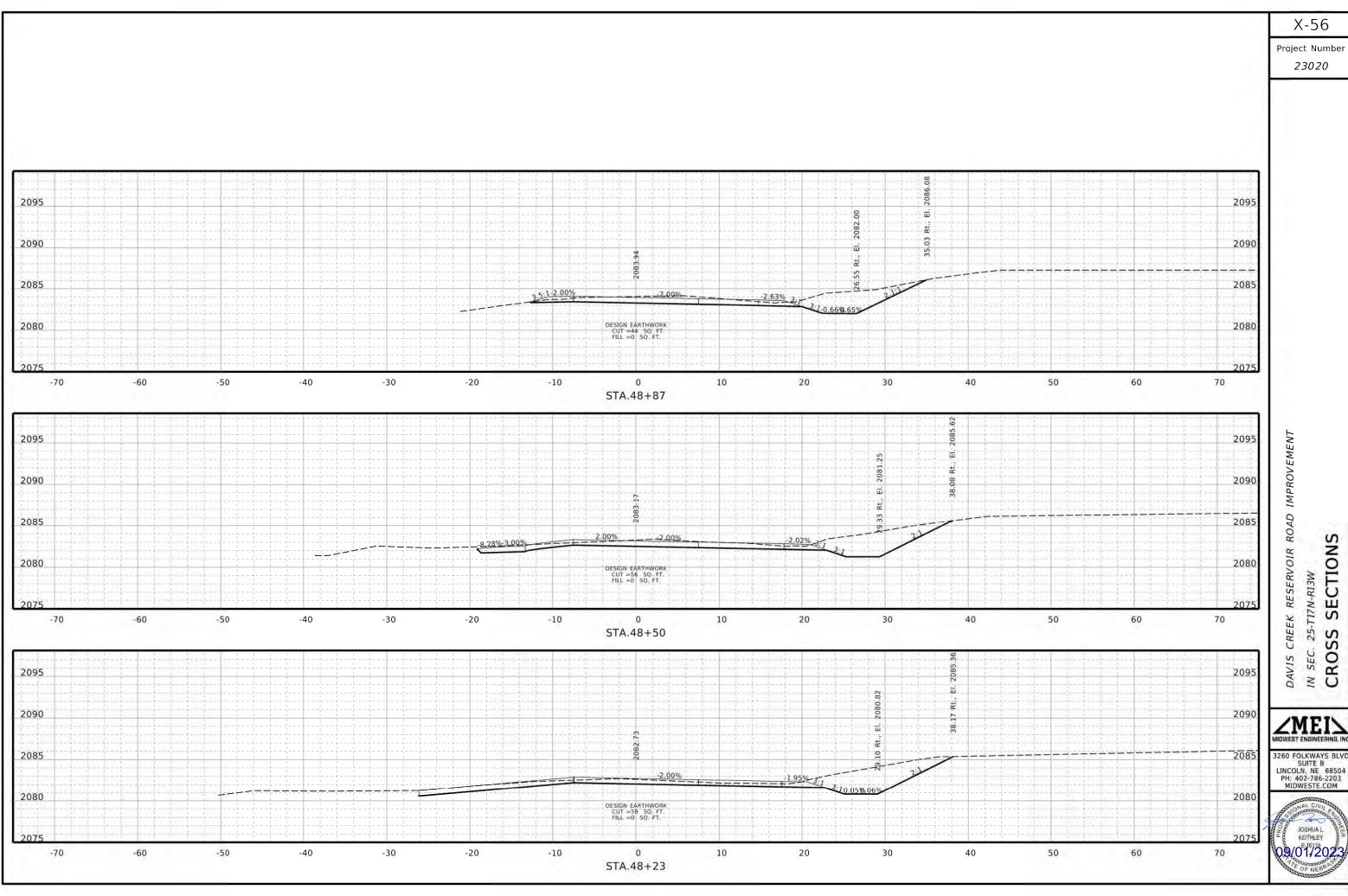


Project Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT
IN SEC. 25-TI7N-R13W
CROSS SECTIONS

MEIN

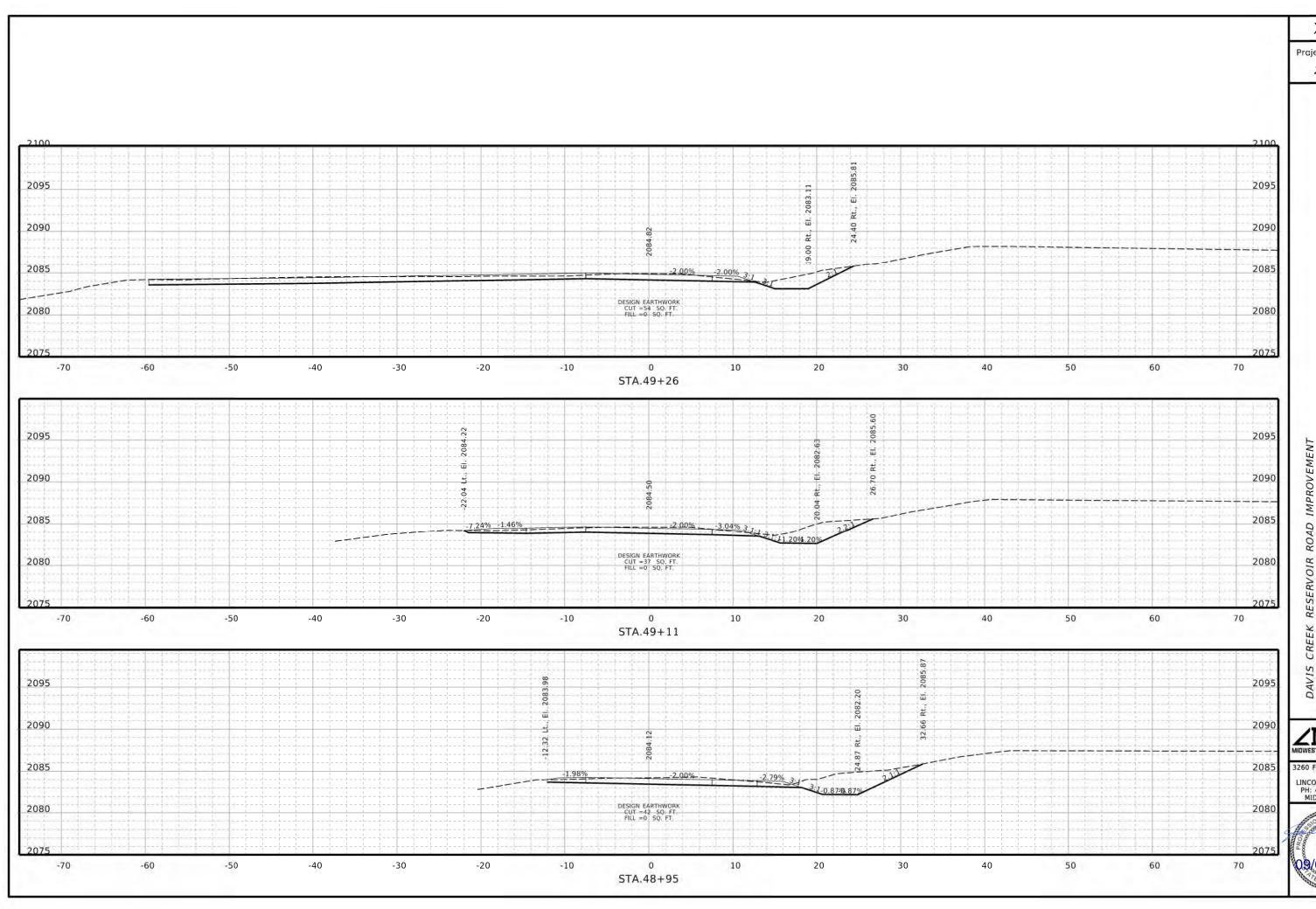




Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-RI3W
CROSS SECTIC

SECTIONS

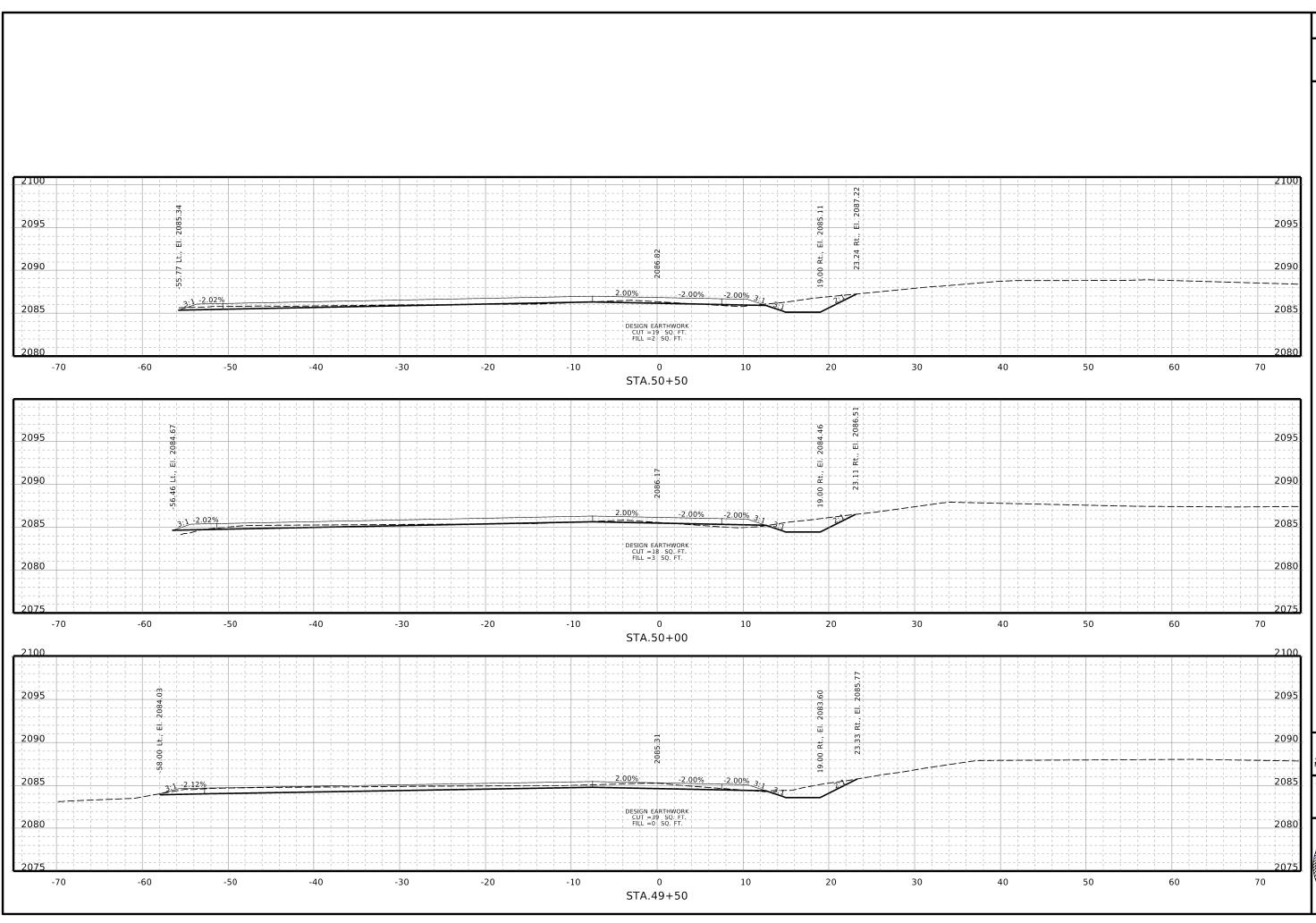


Praject Number 23020

DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TI7N-RI3W
CROSS SECTION

SECTIONS

3260 FOLKWAYS BLVD SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM



Project Number 23020

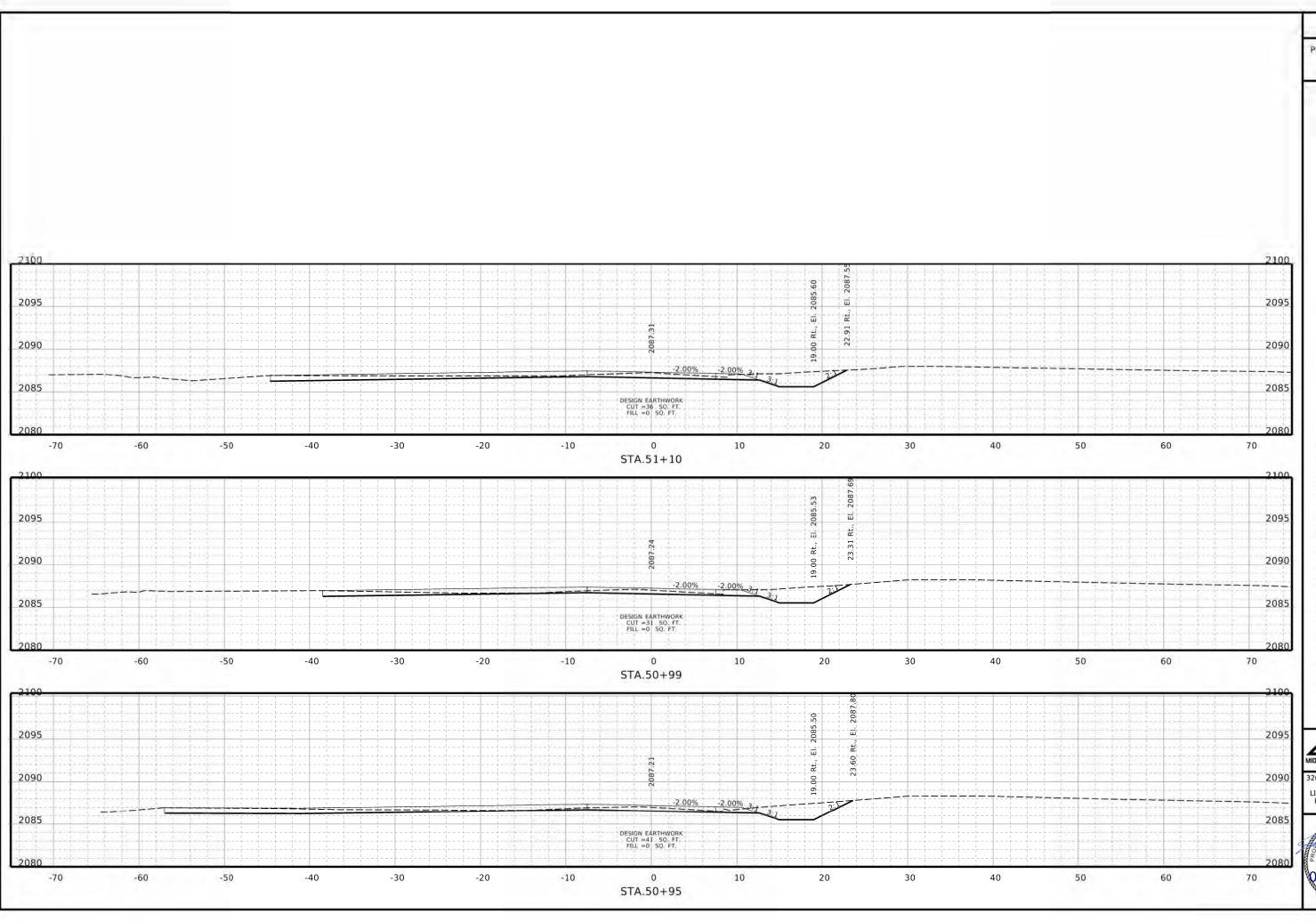
DAVIS CREEK RESERVOIR ROAD IMPROVEMENT

IN SEC. 25-TITN-RI3W
CROSS SECTIONS



3260 FOLKWAYS BLVD SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM





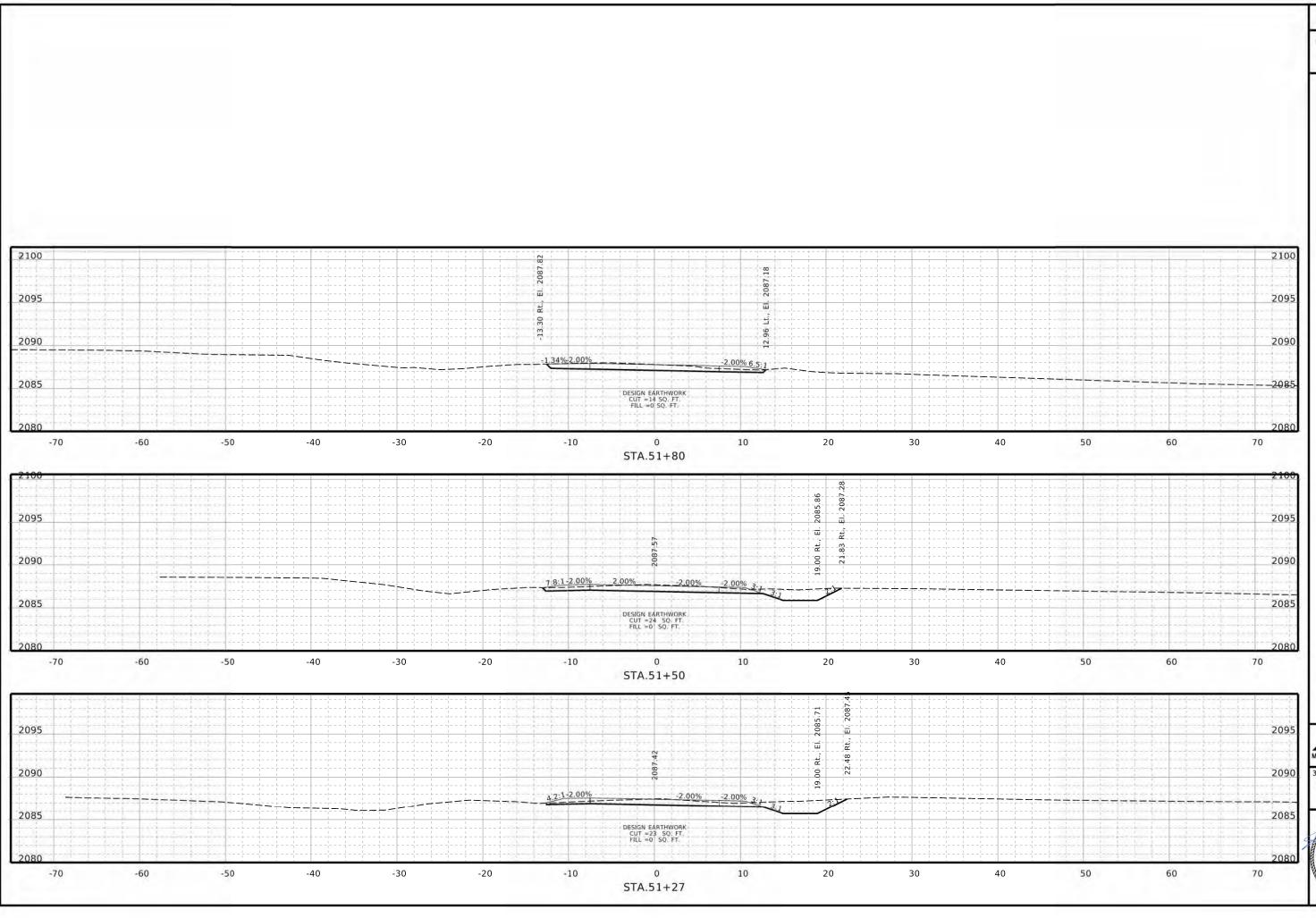
Project Number 23020

> DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RIBW
> CROSS SECTIO

SECTIONS

3260 FOLKWAYS BLVD SUITE B LINCOLN, NE 68504 PH: 402-786-2203 MIDWESTE.COM





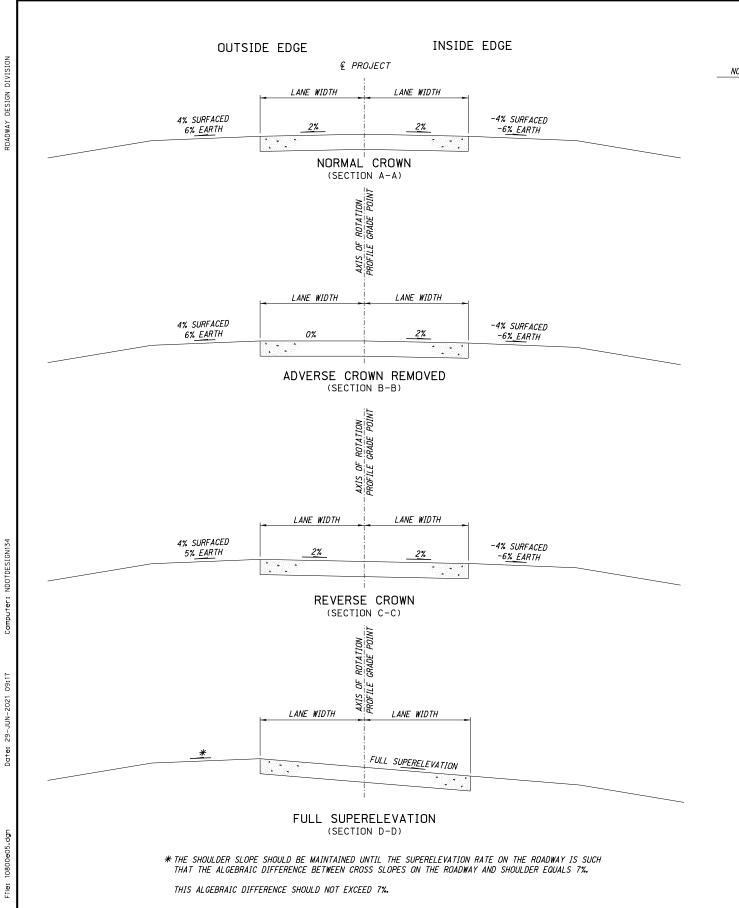
Project Number 23020

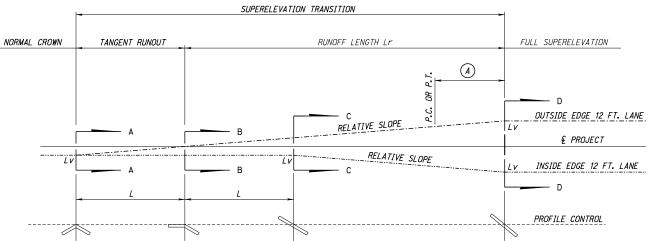
DAVIS CREEK RESERVOIR ROAD IMPROVEMENT IN SEC. 25-TITN-RI3W

CROSS SECTIONS

MEIS MIDWEST ENGINEERING, INC







#### DIAGRAMMATIC PROFILE SHOWING METHOD OF ATTAINING SUPERELEVATION

NOTES:

e = SUPERELEVATION RATE AS SHOWN IN THE PLANS (IN %)

FOR A 28 FT. TOP SYSTEM THE SUPERELEVATION RATE FOR THE 2 FT. SURFACED SHOULDER WILL BE THE SAME AS FOR THE THRU LANE.

AT POINTS MARKED "Lv" IT MAY BE NECESSARY TO INSERT A SHORT CONVENIENT LENGTH OF PARABOLIC CURVE TO ELIMINATE THE SHARP BREAK IN THE STRAIGHT LINE TRANSITION.

L = 12 FT. (WIDTH OF ROADWAY BEING ROTATED) × 0.02 (CHANGE IN ROADWAY CROSS-SLOPE) × RELATIVE SLOPE. Lr = 12 FT. (WIDTH OF ROADWAY BEING ROTATED) × e/100 (FULL SUPERELEVATION) × RELATIVE SLOPE.

FOR A 14 FT. LANE WIDTH L AND Lr SHOULD BE COMPUTED USING THE 12 FT. DRIVING LANE WIDTH.

(A) 60% TO 90% OF THE RUNOFF LENGTH SHOULD BE PLACED ON THE TANGENT.

DESIGN SPEED (mph)	MAXIMUM RELATIVE SLOPE
50	200:1
55	213:1
60	222:1
65	233:1
70	250 <b>:</b> 1

R5	JAN. 18	NDOR BORDER TO NDOT BORDER
R4	OCT. 10	RUNOFF PLACEMENT
R3	SEP. 07	RELATIVE SLOPE TABLE
REV. NO.	DATE	DESCRIPTION OF REVISION

NEBRASKA DEPARTMENT OF TRANSPORTATION STANDARD PLAN NO. 108-R5

SUPERELEVATION PLAN

FOR CONCRETE AND BITUMINOUS SURFACING

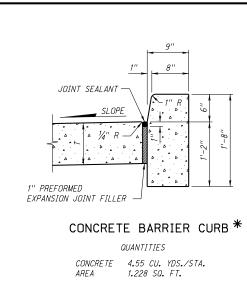
ACCEPTED BY FHWA FOR USE ON THE NATIONAL HIGHWAY SYSTEM:

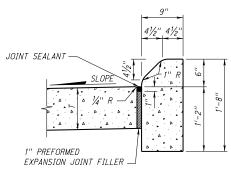


DATE

ORIGINAL:
JULY 30, 1974
DATE







CONCRETE MEDIAN CURB \*

QUANTITIES

1.192 SQ. FT.

CONCRETE 4.42 CU. YDS./STA.

NOTE: \* ONE INCH PREFORMED EXPANSION JOINT FILLER SHALL BE PLACED AT INTERVALS OF NOT MORE THAN 100 FEET THRU CONCRETE BARRIER CURB, CONCRETE MEDIAN CURB, AND CONCRETE CURB, TYPE I.

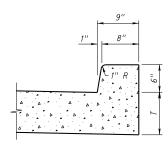
AREA

# SLOPE 1" PREFORMED EXPANSION JOINT FILLER

JOINT SEALANT

## CONCRETE CURB, \* TYPE I

QUANTITIES CONCRETE 5.22 CU. YDS./STA. 1.408 SQ. FT.

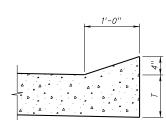


NOTE: MAY BE USED WHEN T IS LESS THAN 1 FOOT.

#### INTEGRAL CONCRETE BARRIER CURB

QUANTITIES

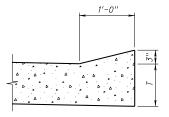
CONCRETE 1.33 CU. YDS./STA. 0.359 SQ. FT.



# INTEGRAL CONCRETE

SLOPING CURB QUANTITIES

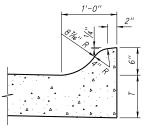
CONCRETE 0.62 CU. YDS./STA. 0.167 SQ. FT. ARF A



#### INTEGRAL CONCRETE SLOPING CURB

QUANTITIES

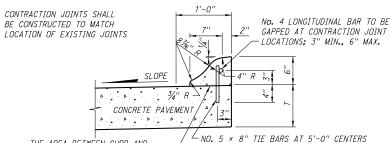
CONCRETE 0.46 CU. YDS./STA.
AREA 0.123 SQ. FT.



#### INTEGRAL CONCRETE CURB

QUANTITIES

CONCRETE 0.89 CU. YDS./STA. 0.239 SQ. FT.



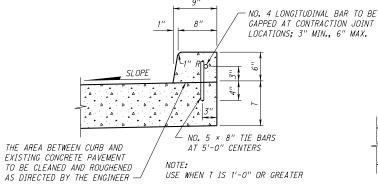
THE AREA BETWEEN CURB AND EXISTING CONCRETE PAVEMENT TO BE CLEANED AND ROUGHENED AS DIRECTED BY THE ENGINEER -

TO BE DRILLED AND GROUTED INTO EXISTING CONCRETE PAVEMENT (WITH APPROVED GROUT)

#### CONCRETE CURB, TYPE II

QUANTITIES

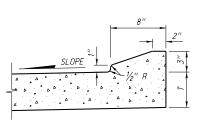
CONCRETE 0.87 CU. YDS./STA. 0.234 SQ. FT.



#### CONCRETE BARRIER CURB ALTERNATE

QUANTITIES

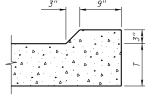
CONCRETE 1.33 CU. YDS./STA. 0.359 SQ. FT.



#### INTEGRAL CONCRETE TRUCK APRON CURB

QUANTITIES

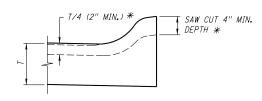
CONCRETE 0.47 CU. YDS./STA. AREA 0.127 SQ. FT.



### EROSION CONTROL CURB

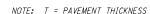
QUANTITIES

CONCRETE 0.81 CU. YDS./STA. 0.219 SQ. FT.



#### CONTRACTION JOINT THRU CURB

\* FOR NON-INTEGRAL CURB THE CONTRACTION JOINTS MAY BE MADE WITH A DOUBLE EDGER WHILE THE CONCRETE IS STILL PLASTIC.



R12	JAN 18	NDOR BORDER TO NDOT BORDER
R11	JUL 15	ADDED TRUCK APRON CURB
R10	FEB 09	MULTIPLE REVISIONS
REV. NO.	DATE	DESCRIPTION OF REVISION

NEBRASKA DEPARTMENT OF TRANSPORTATION STANDARD PLAN NO. 301-R12

## PAVEMENT DETAILS



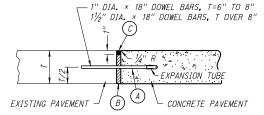
ACCEPTED BY FHWA FOR USE ON THE

ORIGINAL: JANUARY 31, 1974

DATE







- (A) GREASE DOWEL BAR ON EXPANSION TUB SIDE
- B) 1" PREFORMED EXPANSION JOINT FILLER

NOTES:

(C) JOINT SEALANT

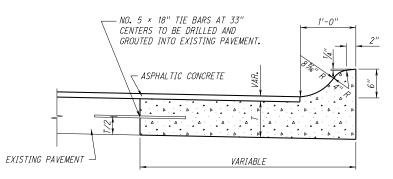
DOWEL BARS SHALL BE DRILLED TO A DEPTH OF 8" INTO EXISTING PAVEMENT AND GROUTED.

DOWEL BARS SHALL BE PLACED AT 1 FOOT CENTERS. THE OUTSIDE DOWEL BAR SHALL BE PLACED 6" FROM THE EDGE OF THE PAVEMENT.

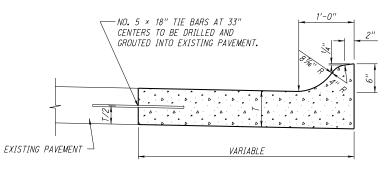
THIS JOINT SHALL BE CONSTRUCTED TRANSVERSE TO THE ROADWAY WHERE THE NEW CONCRETE ABUTS THE EXISTING CONCRETE.

DOWEL BARS SHALL BE PLACED PARALLEL TO THE ROADWAY & AND TO THE ROAD BED.

#### **EXPANSION JOINT** (SUBSIDIARY)

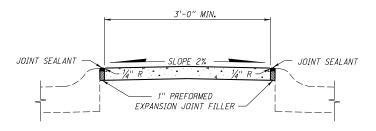


#### CONCRETE BASE COURSE W/INTEGRAL CURB



THE FOLLOWING NOTE IS TYPICAL FOR CONCRETE BASE COURSE WITH INTEGRAL CURB AND CONCRETE PAVEMENT WIDENING: CONTRACTION AND EXPANSION JOINTS SHALL BE CONSTRUCTED TO MATCH LOCATIONS OF EXISTING JOINTS.

CONCRETE PAVEMENT WIDENING



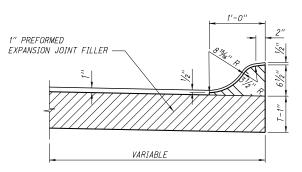
#### CONCRETE MEDIAN SURFACING

ONE INCH PREFORMED EXPANSION JOINT FILLER SHALL BE PLACED ACROSS THE FULL WIDTH OF THE MEDIAN SURFACING AT INTERVALS OF NOT MORE THAN 49 FEET.

LONGITUDINAL JOINTS ONE INCH DEEP SHALL BE MADE IN ALL MEDIANS WHEN SURFACING WIDTH IS 16 FEET OR GREATER.

TRANSVERSE JOINTS ONE INCH DEEP SHALL BE MADE IN ALL MEDIANS AT INTERVALS OF NOT MORE THAN 8 FEET.

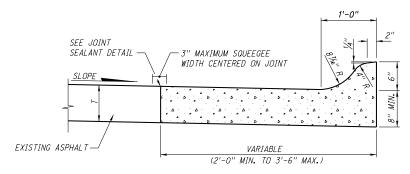
TRANSVERSE AND LONGITUDINAL JOINTS SHALL NOT BE FILLED.



ONE INCH PREFORMED EXPANSION JOINT FILLER SHALL BE PLACED AT INTERSECTION RETURNS AND WHERE SHOWN ON THE PLANS. TRANSVERSE JOINTS SHALL BE PLACED EVERY 8 FEET OR WHERE SHOWN ON THE PLANS.

RECESS THE EXPANSION JOINT FILLER 1/2" FROM THE TOP SURFACE OF THE CURB UNDER CONSTRUCTION

#### DETAIL FOR CUTTING & PLACEMENT OF EXPANSION JOINT FILLER

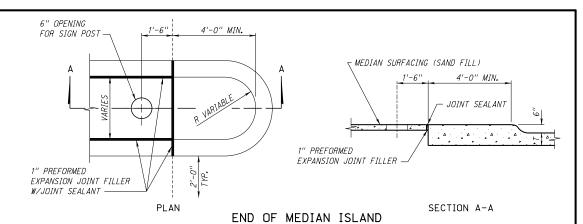


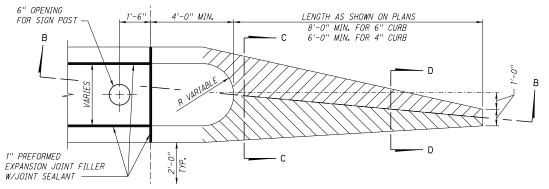
#### COMBINATION CONCRETE CURB & GUTTER

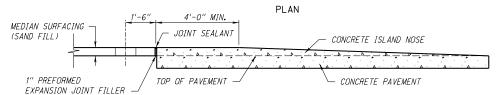
NOTE:

TRANSVERSE JOINTS SHALL BE PLACED EVERY 8 FEET AND JOINTS SHALL BE PLACED AT EACH HEADER, 2-NO. 5 × 18" TIE BARS ARE TO BE USED.

PLACE 1" PREFORMED EXPANSION JOINT FILLER AND SEAL AT THE RETURN OF RADIUS AT INTERSECTIONS.





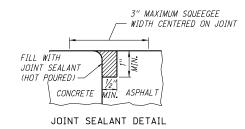


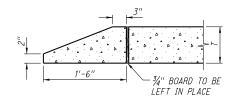
SECTION B-B



#### CONCRETE ISLAND NOSE

NOTE: EXISTING CONCRETE PAVEMENT IS TO BE REMOVED TO BUILD CONCRETE ISLAND NOSE.





CONCRETE HEADER

#### NOTE: T = PAVEMENT THICKNESS

R12	JAN 18	NDOR BORDER TO NDOT BORDER
R11	JUL 15	ADDED TRUCK APRON CURB
R10	FEB 09	MULTIPLE REVISIONS
REV. NO.	DATE	DESCRIPTION OF REVISION

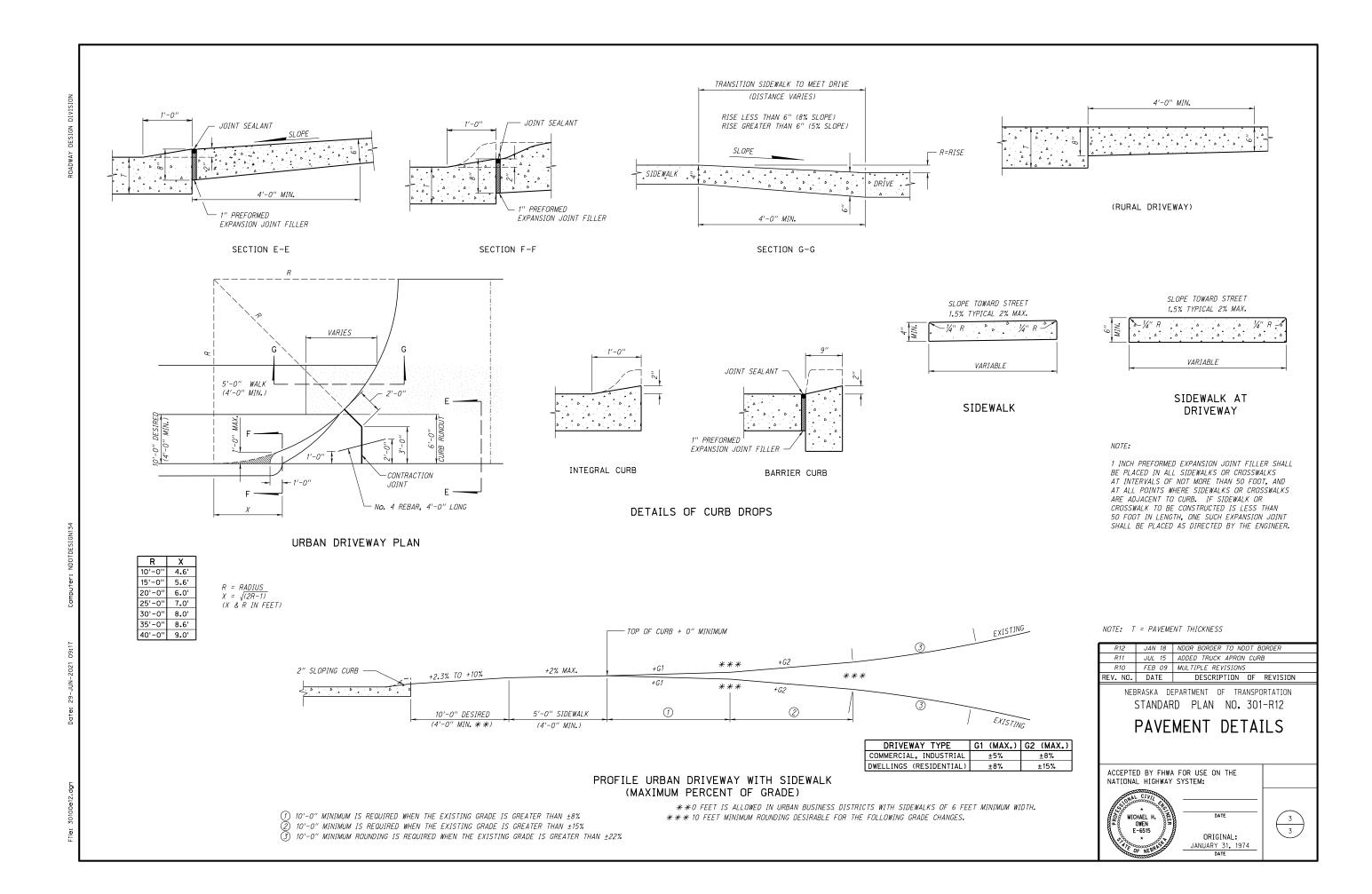
NEBRASKA DEPARTMENT OF TRANSPORTATION STANDARD PLAN NO. 301-R12

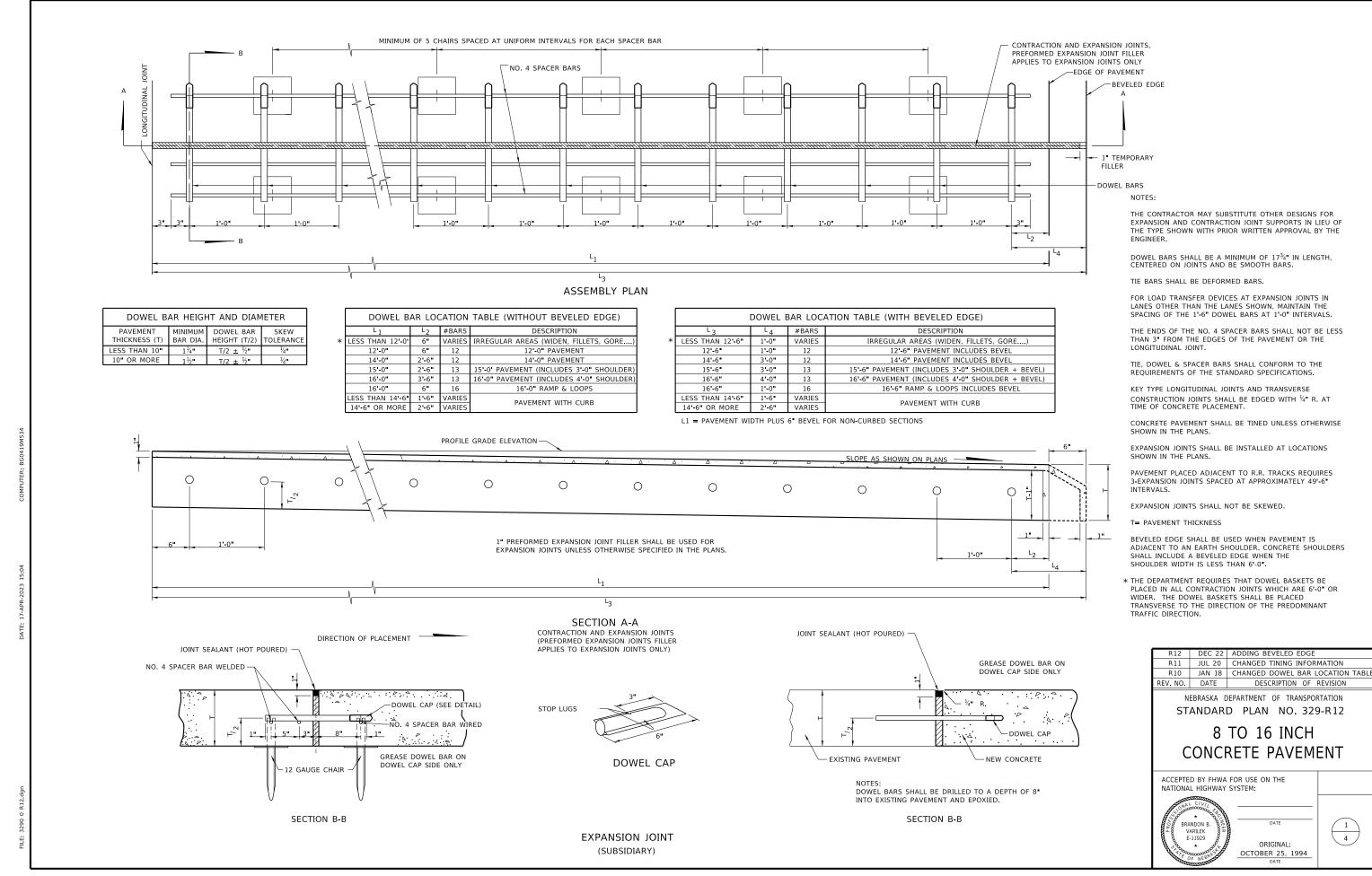
## PAVEMENT DETAILS

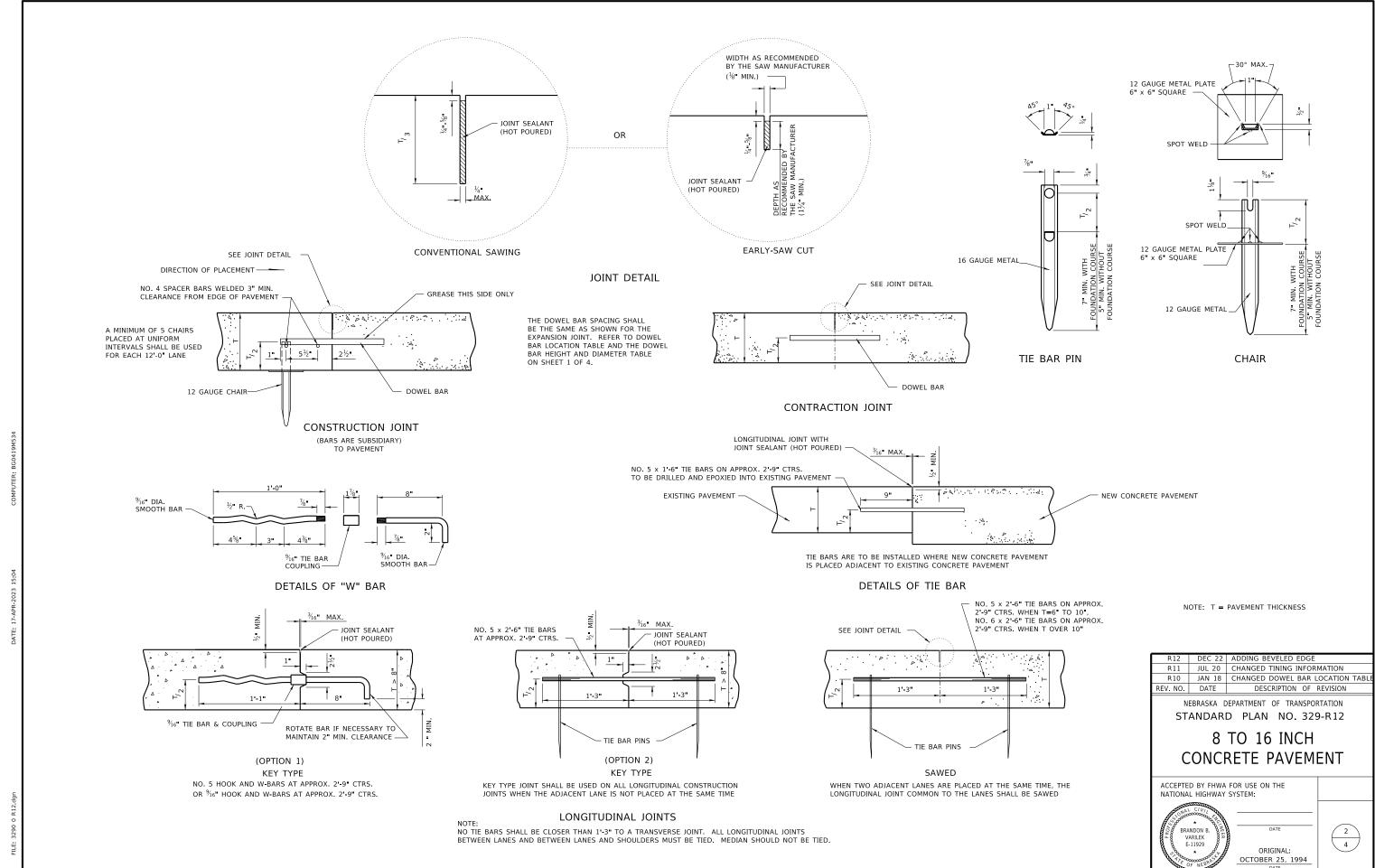


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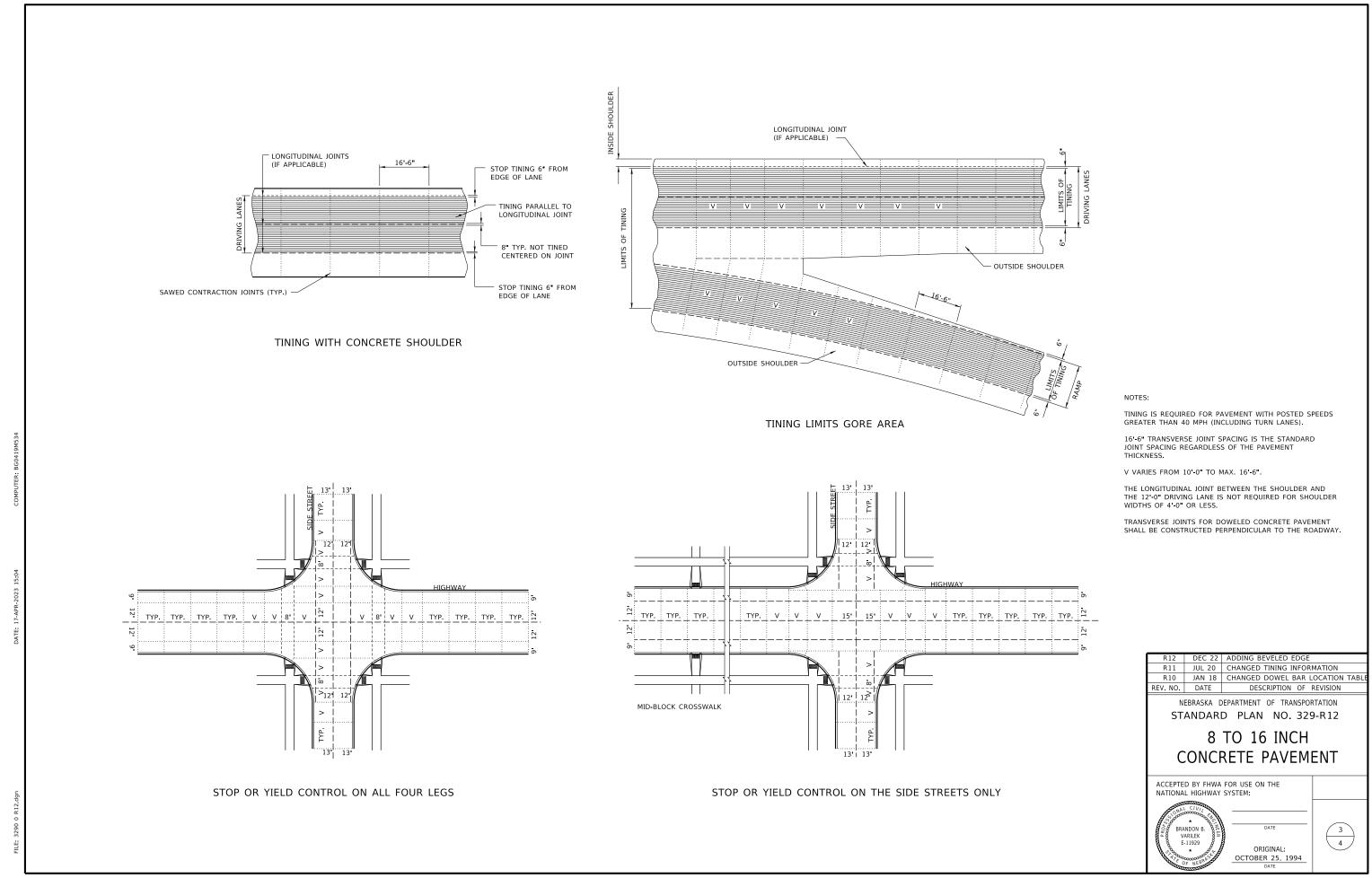
3



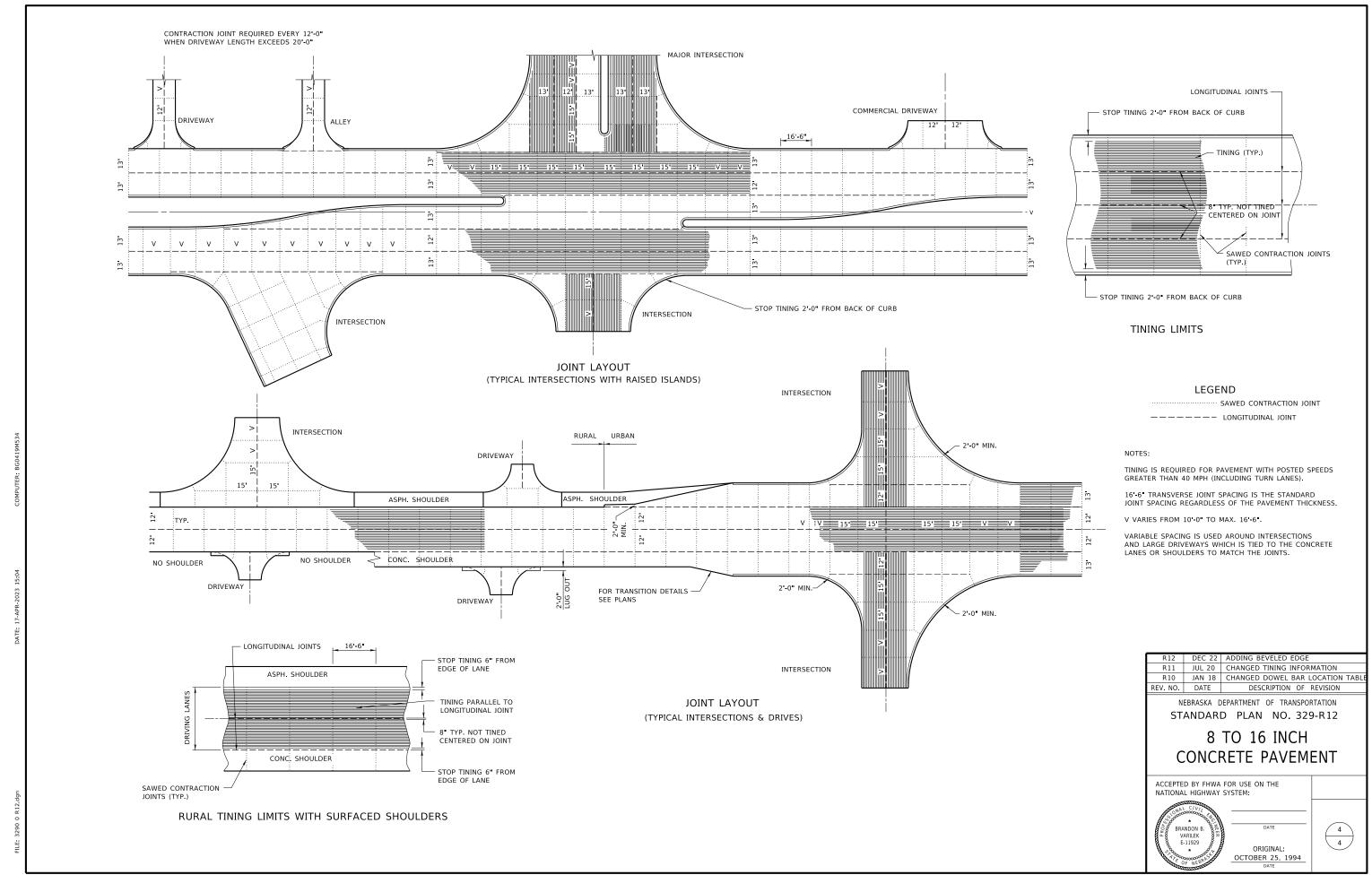




Effective Letting Date: JUNE 2023

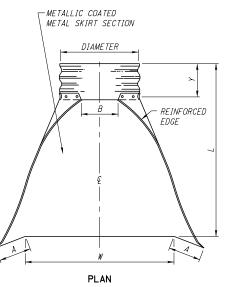


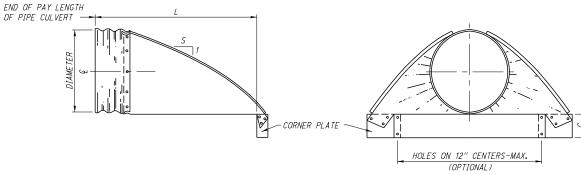
Effective Letting Date: JUNE 2023



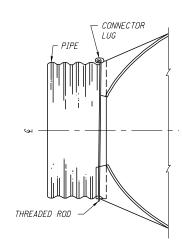
DIDE				NOMIN	AL DIM	ENSION	S	
PIPE DIA.	GAUGE	L ± 6"	₩ ±2"	A MIN.	B MAX.	C MIN.	Y ± 4½"	S APPROX.
12"	16	6'-0%"	2'-0"	43/4"	6"	6"	4'-37/8"	21/2
15"	16	6'-1"	2'-6"	6"	8"	6"	3'-11"	21/2
18"	16	6'-1"	3'-0"	7"	10"	6"	3'-8"	21/2
21"	16	6'-1"	3'-8"	81/4"	1'-0"	6"	3'-1"	21/2
24"	16	6'-11/2"	4'-0"	9"	1'-1"	6"	2'-81/2"	21/2
30"	14	6'-13/4"	5'-0"	11"	1'-4"	6"	1'-103/4"	21/2
36"	14	8'-13/4"	6'-0"	1'-2"	1'-7"	6"	3'-13/4"	21/2
42"	12	8'-2"	7'-0"	1'-4"	1'-10"	6"	2'-5"	21/2
48"	12	8'-2"	7'-6"	1'-6"	2'-3"	6"	1'-8''	21/4
54"	12	8'-4"	8'-6"	1'-6"	2'-6"	6"	1'-4"	2
60"	12	8'-3"	9'-6"	1'-6"	2'-9"	6"	1'-0''	13/4
66"	12	8'-3"	10'-0"	1'-6"	3'-0"	6"	1'-0''	11/2
72"	12	8'-3"	10'-6"	1'-6"	3'-3"	6"	1'-0''	11/3
78''	12	8'-3"	11'-0"	1'-6"	3'-6"	6"	1'-0''	11/4
84"	12	8'-3"	11'-6"	1'-6"	3'-9"	6"	1'-0''	11/6

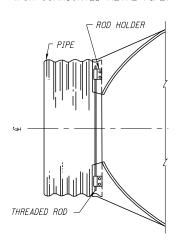
**SECTION** 

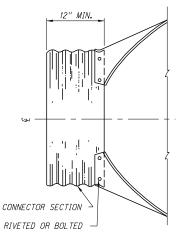




# DETAILS OF METAL FLARED END SECTION (FOR CORRUGATED METAL PIPE)





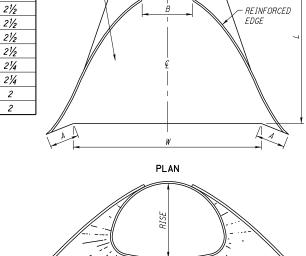


ELEVATION

#### TYPICAL CONNECTIONS

FOR CORRUGATED METAL PIPE DIAMETERS OF 12" TO 24" INCLUSIVE AND CORRUGATED METAL PIPE-ARCHES WITH RISE OF 11" TO 18" INCLUSIVE, THE SKIRT SECTION MAY BE ATTACHED WITH A 1" WIDE, 12 GAUGE METAL CONNECTOR STRAP AND  $\frac{1}{2}$ " × 6" BOLT AND NUT. THIS STRAP MAY BE USED ON PIPE WITH ANNULAR ENDS ONLY.

FOUT						NOMIN	AL DIM	ENSION	S	
EQUIV. DIA.	SPAN	RISE	GAUGE	⊥ - ± 6"	W MIN.	A MIN.	B MAX.	C MIN.	Y ± 4½"	S APPROX.
15"	17"	13"	16	6'-0"	2'-6"	41/2"	9"	6"	4'-5"	21/2
18"	21''	15"	16	6'-0"	3'-0"	5½"	10"	6"	4'-1"	21/2
21"	24"	18''	16	6'-0"	3'-6"	61/4"	111/2"	6"	3'-8"	21/2
24"	28"	20"	16	6'-0"	4'-0"	7"	1'-2"	6"	3'-41/2"	21/2
30"	35"	24"	14	8'-0"	5'-0"	83/4"	1'-4"	6"	4'-91/2"	21/2
36"	42"	29"	14	8'-0"	6'-3"	103/4"	1'-51/2"	6"	4'-2"	21/2
42"	49"	33"	12	8'-0"	7'-1"	1'-01/4"	1'-8"	6"	3'-7"	21/2
48"	57"	38"	12	8'-0"	7'-6"	1'-2"	2'-3"	6"	2'-9"	21/2
54"	64"	43"	12	8'-0"	8'-6"	1'-33/4"	2'-6"	6"	2'-2"	21/4
60"	71''	47"	12	8'-0"	9'-6"	1'-51/4"	2'-9"	6"	1'-7"	21/4
66"	77"	52"	12	8'-0"	10'-6"	1'-6"	3'-0"	6"	1'-7"	2
72"	83"	57"	12	8'-0"	11'-6"	1'-6"	3'-3"	6"	1'-7"	2



HOLES ON 12" CENTERS-MAX.
(OPTIONAL)

ELEVATION

METALLIC COATED METAL SKIRT SECTION

DETAILS OF METAL FLARED END SECTION
(FOR CORUGATED METAL PIPE-ARCH)

CORNER PLATE

NOTES:

CONNECTOR STRAP, STIFFENER ANGLES AND MISCELLANEOUS HARDWARE SHALL BE METALLIC COATED.

SECTION

THE "Y" LENGTH MAY BE FABRICATED AS PART OF THE CULVERT.

END OF PAY LENGTH OF PIPE CULVERT —

CONNECTOR SECTIONS AND CORNER PLATES FOR CORRUGATED METAL PIPE AND PIPE-ARCH FLARED END SECTIONS SHALL BE METALLIC COATED AND OF THE SAME GAUGE AS SKIRTS AND EACH SHALL BE METALLIC COATED.

SKIRT SECTION FOR CORRUGATED METAL PIPE DIA. OF 12" TO 24" INCLUSIVE SHALL BE MADE IN ONE PIECE.

SKIRT SECTION FOR CORRUGATED METAL PIPE-ARCHES WITH RISE OF 11" TO 22" INCLUSIVE SHALL BE MADE IN ONE PIECE.

SKIRT SECTION FOR CORRUGATED METAL PIPE DIA. OF 30" TO 54" INCLUSIVE AND CORRUGATED METAL PIPE-ARCHES WITH RISE OF 27" TO 40" INCLUSIVE MAY BE MADE FROM TWO SHEETS JOINED BY RIVETING OR BOLTING ON CENTERLINE.

SKIRT SECTION OF CORRUGATED METAL PIPE DIA. OF 60" AND LARGER, AND CORRUGATED METAL PIPE-ARCHES WITH RISE OF 44" AND LARGER SHALL BE MADE FROM THREE SHEETS JOINED BY RIVETING OR BOLTING AT EOUAL DISTANCES FROM CENTERLINE. THE CENTER PANEL SHALL BE FURNISHED IN 10 GAUGE MATERIAL AND THE WIDTH OF THE CENTER PANEL SHALL BE GREATER THAN 20% OF THE PIPE PERIPHERY.

MULTIPLE SHEET SKIRT SECTIONS SHALL HAVE 2" MIN. LAP SEAMS. BOLTS OR RIVETS SHALL BE  $\frac{3}{6}$ " DIA. (MIN.) AND ON 6" CENTERS (MAX.).

TYPICAL CONNECTIONS SHOWN MAY BE USED FOR HELICAL CORRUGATED METAL PIPE.

FOR SKIRT SECTIONS OF 60" DIA. PIPE AND LARGER, AND CORRUGATED METAL PIPE-ARCHES WITH A RISE OF 49" AND LARGER, REINFORCED EDGES TO BE SUPPLEMENTED WITH STIFFENER ANGLES PLACED JUST BELOW THE REINFORCED EDGES ON THE OUTSIDE OF THE SKIRT SECTION. THE ANGLES WILL BE 2" × 2" × ¼". THE ANGLES TO BE ATTACHED BY ¾" DIA. (MIN.) BOLTS AND NUTS AND ON 6" CENTERS (MAX.).

R4	JAN 18	NDOR BORDER TO NDOT BORDER
R3	AUG 99	CHANGED NOTES
R2	MAR 89	SPAN. RISE SIZES FOR C.M. PIPE-ARCH
REV. NO.	DATE	DESCRIPTION OF REVISION

NEBRASKA DEPARTMENT OF TRANSPORTATION STANDARD PLAN NO. 410-R4

# FLARED END SECTIONS FOR CULVERT PIPES

ACCEPTED BY FHWA FOR USE ON THE NATIONAL HIGHWAY SYSTEM:



DATE

ORIGINAL: FEBRUARY 22, 1974



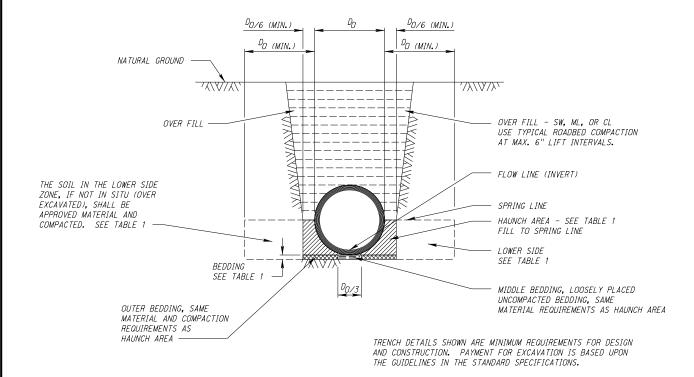
#### TABLE 1 - CONCRETE STANDARD INSTALLATIONS, SOILS AND MINIMUM COMPACTION REQUIREMENTS

INSTALLATION TYPE	BEDDING THICKNESS	HAUNCH AND OUTER BEDDING	LOWER SIDE
TYPE 1	De contraction	95% SW	90% SW, 95% ML, 100% CL, OR NATURAL SOILS OF EQUAL FIRMNESS
TYPE 2	DO/24 MINIMUM, NOT LESS THAN 3" IF ROCK FOUNDATION,USE DO/12 MINIMUM, NOT LESS THAN 6".	90% SW OR 95% ML	85% SW, 90% ML, 95% CL, OR NATURAL SOILS OF EQUAL FIRMNESS
*TYPE 3	THAN 0.	85% SW, 90% ML, OR 95% CL	85% SW, 90% ML, 95% CL, OR NATURAL SOILS OF EQUAL FIRMNESS

TABLE 1 NOTES:

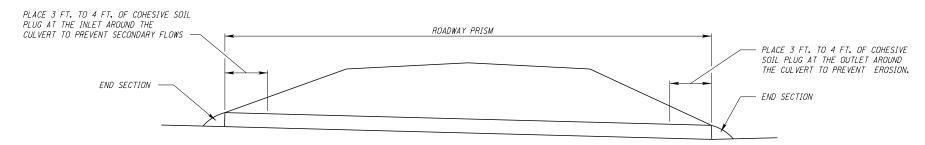
\* THE TYPE 3 INSTALLATION (SHADED) IN TABLE 4 IS THE NDOR MINIMUM STANDARD, USING EITHER A SHAPED TRENCH ACCORDING TO THE STANDARD SPECIFICATIONS, OR AT THE OPTION OF THE CONTRACTOR, THE BEDDING WITH COMPACTIONS AS SHOWN.

MAXIMUM FILL HEIGHTS FOR THE TYPE 1, 2, AND 3 INSTALLATIONS ARE



TRENCHES SHALL BE EXCAVATED IN ACCORDANCE WITH APPROVED SAFETY PRACTICE.

#### TYPICAL TRENCH INSTALLATION



#### LIMITS OF BEDDING AND BACKFILL

EXCAVATION, BEDDING AND EMBANKMENT SEQUENCE:

#### TRENCH INSTALLATION:

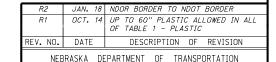
- (A) DETERMINE THE FLOW LINE AND TRENCH BOTTOM ELEVATIONS.
- (B) DETERMINE THE SHAPE OF TRENCH. DECIDE IF SHORING IS NEEDED. CONTRACTOR IS ULTIMATELY RESPONSIBLE FOR THE SAFETY OF ALL WORKERS, EQUIPMENT AND MATERIALS IN THE TRENCH.
- (C) PLACE THE BEDDING MATERIAL (SEE CONCRETE TABLE 1) LOOSELY.
- (D) PLACE PIPE ON THE BEDDING AND COMPACT OUTER BEDDING, (SEE TABLE 1).
- (E) PLACE AND COMPACT THE LOWER SIDE, HAUNCH AND OVERFILL MATERIAL AT 6 IN. INTERVALS.

#### EMBANKMENT INSTALLATION:

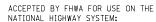
- (A) DETERMINE THE FLOW LINE AND SPRING LINE ELEVATION.
- (B) IF FLOW LINE IS ABOVE THE NATURAL GROUND, PLACE AN EMBANKMENT AT LEAST 3DO WIDE WITH 3:1 FORESLOPES OR FLATTER AT SPRING LINE ELEVATION. COMPACTED AT ROADBED REQUIRED COMPACTION.
- (C) IF THE FLOW LINE IS BELOW THE NATURAL GROUND BUT THE SPRING LINE IS ABOVE THE NATURAL GROUND, PLACE THE EMBANKMENT SIMILAR TO THE ONE IN STEP B.
- (D) EXCAVATE TO PROPER ELEVATION.
- (E) PLACE BEDDING MATERIAL (SEE TABLE 1) LOOSELY.
- (F) PLACE THE PIPE ON THE BEDDING MATERIAL AND COMPACT OUTER BEDDING MATERIAL (SEE CONCRETE TABLE 1).
- (G) PLACE AND COMPACT THE HAUNCH, LOWER SIDE AND OVERFILL MATERIAL AT 6 IN. INTERVALS.

#### NOTES FOR TRENCH INSTALLATIONS:

- 1. COMPACTION AND SOIL SYMBOLS, I.E. 95% SW, REFER TO SW SOIL MATERIAL WITH MINIMUM STANDARD PROCTOR COMPACTION OF 95%
- 2. THE TRENCH TOP ELEVATION SHALL BE NO LOWER THAN 1 FT. BELOW THE BOTTOM OF THE PAVEMENT BASE MATERIAL.
- 3. SOIL IN BEDDING AND HAUNCH ZONES SHALL BE COMPACTED TO AT LEAST THE SAME COMPACTION AS SPECIFIED FOR THE MAJORITY OF SOIL IN THE BACKFILL ZONES.
- 4. THE TRENCH WIDTH SHALL BE WIDER THAN SHOWN IF REQUIRED FOR ADEQUATE SPACE TO ATTAIN THE SPECIFIED COMPACTION IN THE HAUNCH AND BEDDING ZONES.
- 5. FOR TRENCH WALLS THAT ARE WITHIN 10 DEGREES OF VERTICAL, THE COMPACTION OR FIRMNESS OF THE SOIL IN THE TRENCH WALLS AND LOWER SIDE ZONE NEED NOT TO BE CONSIDERED.
- 6. FOR TRENCH WALLS WITH GREATER THAN 10 DEGREE SLOPES THAT CONSIST OF EMBANKMENT, THE LOWER SIDE SHALL BE COMPACTED TO AT LEAST THE SAME COMPACTION AS SPECIFIED FOR THE SOIL IN THE BACKFILL ZONE.



STANDARD PLAN NO. 411-R2 BEDDING AND BACKFILL REQUIREMENTS FOR CONCRETE PIPE









#### TABLE 1 - CONCRETE STANDARD INSTALLATIONS, SOILS AND MINIMUM COMPACTION REQUIREMENTS

INSTALLATION TYPE	BEDDING THICKNESS	HAUNCH AND OUTER BEDDING	LOWER SIDE
TYPE 1	D	95% SW	90% SW, 95% ML OR 100% CL
TYPE 2	DO/24 MINIMUM, NOT LESS THAN 3" IF ROCK FOUNDATION,USE DO/12 MINIMUM, NOT LESS THAN 6".	90% SW OR 95% ML	85% SW, 90% ML OR 95% CL
*TYPE 3	THAN 6.	85% SW, 90% ML, OR 95% CL	85% SW, 90% ML OR 95% CL

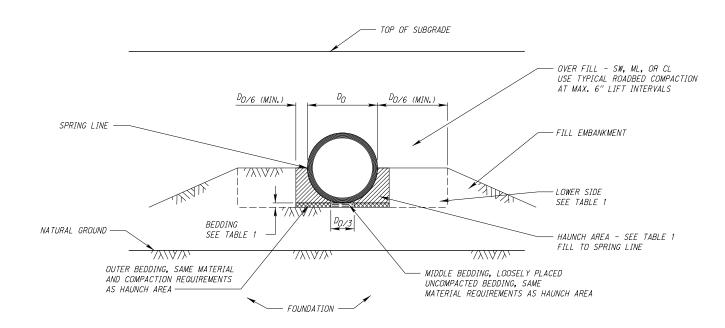
TABLE 1 NOTES:

\*\* THE TYPE 3 INSTALLATION (SHADED) IN TABLE 4 IS THE NDOR MINIMUM STANDARD, USING EITHER A SHAPED TRENCH ACCORDING TO THE STANDARD SPECIFICATIONS, OR AT THE OPTION OF THE CONTRACTOR, THE BEDDING WITH COMPACTIONS AS SHOWN.

MAXIMUM FILL HEIGHTS FOR THE TYPE 1, 2, AND 3 INSTALLATIONS ARE SHOWN IN TABLE 4

TABLE 2 - CONCRETE PIPE DIMENSIONS

NOMINAL PIPE		STANDARD OUTSIDE PIPE DIAMETER, DO (SPAN)				
(INCHES)	ROUND PIPE	ARCH PIPE	H. ELLIP. PIPE	V. ELLIP. PIPE		
15	19.5	22.5				
18	23	27	28.5			
21	26.5	31.5				
24	30	34.5	36.5			
27	33.5		41			
30	37	43.25	45.5			
36	44	51.75	54	38		
42	51	60.13	63	44		
48	58	68.5	71	49		
54	65	76	80	55		
60	72	85	89	61		
66	79		97	67		
72	86	102	106	73		
78	93		114	79		
84	100	118	123	85		
90	107					
96	114					
102	121					
108	128					



TYPICAL EMBANKMENT INSTALLATION

# TABLE 3 SOIL CLASSIFICATION FOR BEDDING & BACKFILL

ASTM SOIL GROUP		PERCENTAGE PASSING SIEVE SIZES				
SYMBOL D 2487	DESCRIPTION		NO. 4	NO. 200		
SW	WELL GRADED SANDS AND GRAVELLY—SANDS: LITTLE OR NO FINES. NON PLASTIC		>50% OF "COURSE FRACTION"	( 5%		
ML	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY-FINE-SANDS, SILTS WITH SLIGHT PLASTICITY	100%	100%	) 50%		
CL	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELY-CLAYS, SANDY-CLAYS, SILTY-CLAYS, LEAN CLAYS		100%	7 50%		

NOTES FOR EMBANKMENT INSTALLATIONS:

- 1. COMPACTION AND SOIL SYMBOLS, I.E. 95% SW, REFER TO SW SOIL MATERIAL WITH A MINIMUM STANDARD PROCTOR COMPACTION OF 95%.
- 2. SOIL IN THE OUTER BEDDING, HAUNCH, AND LOWER SIDE ZONES, EXCEPT WITHIN THE <sup>D</sup>O/3 MIDDLE BEDDING, SHALL BE COMPACTED TO AT LEAST THE SAME COMPACTION AS THE MAJORITY OF THE SOIL IN THE OVERFILL ZONES.

#### 3. SURTRENCHES

- 3.1 A SUBTRENCH IS DEFINED AS A TRENCH WITH ITS TOP AT AN ELEVATION LOWER THAN 1 FT. BELOW THE BOTTOM OF THE PAVEMENT BASE MATERIAL.
- 3.2 THE MINIMUM WIDTH OF A SUBTRENCH SHALL BE 1.33DD, OR WIDER IF REQUIRED FOR ADEQUATE SPACE TO ATTAIN THE SPECIFIED COMPACTION IN THE HAUNCH AND BEDDING ZONES.
- 3.3 FOR SUBTRENCHES WITH WALLS OF NATURAL SOIL, ANY PORTION OF THE LOWER SIDE ZONE IN THE SUBTRENCH WALL SHALL BE AT LEAST AS FIRM AS AN EQUIVALENT SOIL PLACED TO THE COMPACTION REQUIREMENTS SPECIFIED FOR THE LOWER SIDE ZONE, AND AS FIRM AS THE MAJORITY OF SOIL IN THE OVERFILL ZONE, OR SHALL BE REMOVED AND REPLACED WITH SOIL COMPACTED TO THE SPECIFIED LEVEL.

GENERAL NOTES:

WHEN IN-SITU LATERAL SOIL RESISTANCE IS NEGLIGIBLE, E.G. PEAT, MUCK, OR HIGHLY EXPANSIVE SOIL, EMBEDMENT SHALL BE PLACED AND COMPACTED AT THE DIRECTION OF THE ENGINEER.

TO PROTECT THE PIPE AND BACKFILL DURING CONSTRUCTION, PROVIDE A MINIMUM OF 36" OF COMPACTED FILL MATERIAL OVER THE TOP OF THE PIPE BEFORE ALLOWING ANY HEAVY EQUIPMENT TO TRAVERSE OVER THE PIPE. EXTREMELY HEAVY EQUIPMENT MAY REQUIRE LARGER COVER AS DETERMINED BY THE CONTRACTOR.

THE PIPE VOLUME SHOULD NOT BE SUBTRACTED FROM THE VOLUME OF EXCAVATION.

THESE DESIGN STANDARDS ARE MINIMUM. IF A MORE RESTRICTIVE DESIGN IS REQUIRED BY THE ENGINEER OR CULVERT MANUFACTURER, THEN THESE STANDARDS SHALL BE MODIFIED. CHANGES TO PAY ITEM QUANTITIES DUE TO UNFORESEEN SITE CONDITIONS SHALL BE CALCULATED AND INCORPORATED INTO THE CONTRACT THRU A CHANGE ORDER.

BOTH ENDS OF THE PIPE SHALL BE SEALED WITH COHESIVE SOIL (AROUND THE PIPE EXTENDING 3 FT. TO 4 FT. FROM EACH END) TO PROTECT AGAINST INFILTRATION AND EROSION.

BEDDING AND BACKFILL MATERIAL IS NOT PAID FOR DIRECTLY, BUT IS SUBSIDIARY TO THE LINEAR FEET OF CULVERT.

BEDDING AND BACKFILL MATERIAL SHALL MEET ASTM D 2487 (SOIL GROUPS AS SHOWN IN TABLE 3).

PERCENT COMPACTION SHALL BE DETERMINED IN ACCORDANCE WITH NDOR STANDARD TEST METHOD T 99.

R2	JAN. 18	NDOR BORDER TO NDOT BORDER
R1	OCT. 14	UP TO 60" PLASTIC ALLOWED IN ALL OF TABLE 1 - PLASTIC
REV. NO.	DATE	DESCRIPTION OF REVISION
NEE	RASKA DE	EPARTMENT OF TRANSPORTATION

STANDARD PLAN NO. 411-R2
BEDDING AND BACKFILL

# REQUIREMENTS FOR CONCRETE PIPE

ACCEPTED BY FHWA FOR USE ON THE NATIONAL HIGHWAY SYSTEM:



DATE

ORIGINAL: JUNE 6, 2008



#### TABLE 1 - CONCRETE STANDARD INSTALLATIONS, SOILS AND MINIMUM COMPACTION REQUIREMENTS

INSTALLATION TYPE	BEDDING THICKNESS	HAUNCH AND OUTER BEDDING	LOWER SIDE
TYPE 1	D	95% SW	90% SW, 95% ML OR 100% CL
TYPE 2	DO/24 MINIMUM, NOT LESS THAN 3" IF ROCK FOUNDATION,USE DO/12 MINIMUM, NOT LESS THAN 6".	90% SW OR 95% ML	85% SW, 90% ML OR 95% CL
*TYPE 3	THAN D.	85% SW, 90% ML, OR 95% CL	85% SW, 90% ML OR 95% CL

NOTES:

\* THE TYPE 3 INSTALLATION (SHADED) IN TABLE 4 IS THE NDOR MINIMUM STANDARD, USING EITHER A SHAPED TRENCH ACCORDING TO THE STANDARD SPECIFICATIONS, OR AT THE OPTION OF THE CONTRACTOR, THE BEDDING WITH COMPACTIONS AS SHOWN

 $\it MAXIMUM$  FILL HEIGHTS FOR THE TYPE 1, 2, AND 3 INSTALLATIONS ARE SHOWN IN TABLE 4.

INSTALLATION TYPE 2 AND TYPE 1 ARE IMPROVED METHODS IN ORDER TO SUPPORT HIGHER FILL HEIGHTS USING CLASS III, IV, AND V CIRCULAR CONCRETE PIPE. INSTALLATION TYPE I WILL PROVIDE THE BEST IN-SITU PERFORMANCE USING GREATER COMPACTION WITH GRANULAR BEDDING AND BACKFILL. THE CONTRACTOR WILL CHOOSE THE INSTALLATION TYPE AND CLASS OF PIPE. ACTUAL PROJECT FILL HEIGHTS MUST BE KNOWN IN ORDER TO USE TABLE 4.

ROUND EQUIVALENT, NON-CIRCULAR PIPE SUCH AS ARCH OR ELLIPTICAL PIPE, MAY BE SELECTED, PROVIDED SUCH PIPE ARE DESIGNED AND MANUFACTURED TO THE SAME D-LOADS AND ULTIMATE STRENGTHS (SEE TABLE 5) AS THE SELECTED CIRCULAR PIPE FROM THE FILL HEIGHT TABLE.

# TABLE 5 D-LOADS FOR CONCRETE PIPE

PIPE CLASS	III	IV	٧
D-LOAD TO PRODUCE A 0.01-IN. CRACK	1350	2000	3000
D-LOAD TO PRODUCE THE ULTIMATE LOAD	2000	3000	3750

NOTES:

LOAD ON PIPE IN POUNDS PER LINEAR FOOT = D-LOAD X INSIDE SPAN IN FEET D-LOAD = TEST LOAD EXPRESSED IN POUNDS-FORCE PER LINEAR FOOT PER FOOT OF DIAMETER

# TABLE 4 MAXIMUM FILL HEIGHTS (FEET) FOR STANDARD DESIGN (AASHTO M 170) ROUND CONCRETE PIPE

		LLATION T OR STANDA		INSTA	LLATION T	YPE 2	INST	ALLATION T	YPE 1
PIPE SIZE (IN.)	CLASS III	CLASS IV	CLASS V	CLASS III	CLASS IV	CLASS V	CLASS III	CLASS IV	CLASS V
15	12	15	21	15	19	26	23	28	40
18	12	17	24	16	22	30	24	32	45
21	13	19	26	16	24	32	25	37	48
24	13	19	26	17	24	33	25	32	45
27	13	17	26	17	21	34	23	26	51
30	12	14	25	15	17	32	20	21	49
36	10	16	24	13	21	31	20	31	47
42	10	15	23	13	19	29	20	29	44
48	10	14	22	13	18	29	20	28	43
54	10	14		13	17		20	27	
60	9	14		12	18		19	28	
66	9	14		12	18		19	28	
72	9	14		12	18		19	28	
78	9			12			19		
84	9			12			19		
90	9			12			20		
96	9			12			19		
102	10			13			20		
108	10			14			22		

TABLE 4 NOTES:

AASHTO M 170 SPECIFICATIONS ARE MODIFIED AS FOLLOWS:

ONLY SINGLE INNER CAGE, CIRCULAR REINFORCING IS ALLOWED FOR CLASS III, 15", 18", 21", AND 24" ROUND RCP AS SHOWN:

PIPE SIZE (IN.)	CLASS	MINIMUM CIRCUMFERENTIAL REINFORCING (IN. <sup>2</sup> /FT. OF PIPE WALL)
15	III	0.08
18	III	0.10
21	III	0.12
24	III	0.14

APPLICABLE SPECIFICATIONS:

AASHTO M 170---ROUND RCP AASHTO M 206---ARCH RCP AASHTO M 207---ELLIPTICAL RCP GENERAL NOTES:

FILL HEIGHTS SHOWN IN TABLE 4 WERE DEVELOPED USING ASCE STANDARDS FOR DIRECT DESIGN OF BURIED PRECAST CONCRETE PIPE, MANUFACTURED IN ACCORDANCE WITH AASHTO M 170
SPECIFICATION REQUIREMENTS (SEE TABLE 4 FOOTNOTE FOR EXCEPTIONS). FILL HEIGHTS SHOWN APPLY ONLY TO ROUND PIPE (UNDER FULL FLOW CONDITIONS), USED UNDER RIGID AND FLEXIBLE PAVEMENT, WITH SOIL OVERFILL WEIGHING 120 POUNDS PER CUBIC FOOT. UNDER SPECIAL CIRCUMSTANCES (WHERE PAVEMENT IS NOT USED AND LIVE LOAD BECOMES CRITICAL, OR DIFFERENT SOIL DENSITY IS ENCOUNTERED, OR THE ONE FOOT MINIMUM CLEARANCE FROM THE BOTTOM OF THE PAVEMENT TO THE TOP OF THE PIPE CANNOT BE MADIFIED. DEEPER FILL HEIGHTS MAY NEED TO BE MODIFIED. DEEPER FILL HEIGHTS MAY BE USED BY SUBMITTING A SPECIAL STANDARD INSTALLATION DIRECT DESIGN (SIDD) FOR NODOR APPROVAL.

CONCRETE PIPE DESIGNS THAT ARE NOT SHOWN IN APPLICABLE AASHTO SPECIFICATIONS WILL BE CONSIDERED SPECIAL DESIGNS THAT MUST BE SUBMITTED TO NDOR FOR APPROVAL.

R2	JAN. 18	NDOR BORDER TO NDOT BORDER
R1	OCT. 14	UP TO 60" PLASTIC ALLOWED IN ALL OF TABLE 1 - PLASTIC
REV. NO.	DATE	DESCRIPTION OF REVISION

NEBRASKA DEPARTMENT OF TRANSPORTATION

STANDARD PLAN NO. 411-R2

## BEDDING AND BACKFILL REQUIREMENTS FOR CONCRETE PIPE

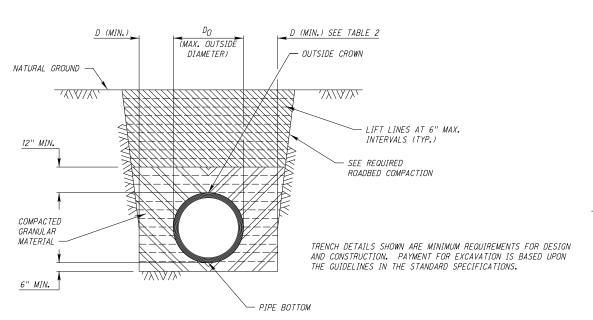
ACCEPTED BY FHWA FOR USE ON THE NATIONAL HIGHWAY SYSTEM:



DATE







TRENCHES SHALL BE EXCAVATED IN ACCORDANCE WITH APPROVED SAFETY PRACTICE.

#### TYPICAL TRENCH INSTALLATION

TABLE 1 - PLASTIC SOIL CLASSIFICATION FOR GRANULAR FILL MATERIAL

SOIL GROUP SYMBOL DESCRIPTION		% PAS	SIZES		
D 2487	DESCRIPTION	1½ IN.	NO. 4	NO. 200	
GW	WELL GRADED GRAVEL AND GRAVEL-SAND MIXTURES; LITTLE OR NO FINES.  POORLY GRADED GRAVEL AND GRAVEL-SAND MIXTURES; LITTLE OR NO FINES.		(50% OF COARSE		
GP			FRACTION		
SW	WELL GRADED SAND AND GRAVEL-SANDS; LITTLE OR NO FINES.	100% ) 50% OF COARS		- (5%	
POORLY GRADED SAND SP AND GRAVEL-SANDS; LITTLE OR NO FINES.			FRACTION		
E.G. GW-GC SP-SM	SAND AND GRAVELS WHICH ARE BORDER LINE BETWEEN CLEAN AND WITH FINES.	100%	VARIES	5% TO 12%	
GM	GM SILTY GRAVEL, GRAVEL- SAND-SILT MIXTURES.		(50% OF COARSE		
GC	CLAYEY-GRAVEL, GRAVEL- SAND-CLAY MIXTURES.	100%	FRACTION	12% TO 50%	
SM	SILTY SANDS, SAND-SILT MIXTURES.		) 50% OF COARSE FRACTION		

TABLE 2 - PLASTIC MINIMUM D (INCHES)

		NCH LATION	EMBANKMENT INSTALLATION	
NOMINAL PIPE DIAMETER (INCHES)	METAL PIPE	PLASTIC PIPE	METAL PIPE	PLASTIC PIPE
15	11	11	15	15
18	12	12	18	18
24	13	13	24	24
30	15	15	24	24
36	17	17	24	24
42	24	24	24	24
48	24	24	24	24
54	24	24	24	24
60	24	24	24	24
66	24		24	
72	24		24	
78	24		24	
84	24		24	

NOTES:

6" MIN.

INSTALLATIONS AS SHOWN ARE REQUIRED UNDER ALL SURFACED ROADWAYS. BEDDING AND BACKFILL FOR DRIVE PIPE OR OTHER PIPE OUTSIDE THE ROADWAY PRISM (OR BACK OF CURB-LINE FOR URBAN PROJECTS) MAY BE INSTALLED USING SUITABLE EXISTING SOIL, PLACED AND COMPACTED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

D (MIN.)

TYPICAL EMBANKMENT INSTALLATION

SEE TABLE 2

WHERE IN-SITU LATERAL SOIL RESISTANCE IS NEGLIGIBLE E.G. PEAT, MUCK, OR HIGHLY EXPANSIVE SOIL, EMBEDMENT SHALL BE PLACED AND COMPACTED AT THE DIRECTION OF THE ENGINEER.

TO PROTECT THE PIPE AND BACKFILL DURING CONSTRUCTION, PROVIDE A MINIMUM OF 36" OF COMPACTED FILL MATERIAL OVER THE TOP OF THE PIPE BEFORE ALLOWING ANY HEAVY EQUIPMENT TO TRAVERSE OVER THE PIPE. EXTREMELY HEAVY EQUIPMENT MAY REQUIRE LARGER COVER AS DETERMINED BY THE CONTRACTOR.

PIPE VOLUME SHOULD NOT BE SUBTRACTED FROM THE VOLUME OF EXCAVATION.

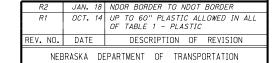
THESE DESIGN STANDARDS ARE MINIMUM. IF A MORE RESTRICTIVE DESIGN IS REQUIRED BY THE ENGINEER OR THE CULVERT MANUFACTURER, THEN THESE STANDARDS SHALL BE MODIFIED. CHANGES TO PAY ITEM QUANTITIES DUE TO UNFORESEEN SITE CONDITIONS SHALL BE CALCULATED AND INCORPORATED INTO THE CONTRACT BY A CHANGE ORDER.

EXPOSED ENDS OF THE PIPE SHALL BE SEALED WITH COHESIVE SOIL (AROUND THE PIPE EXTENDING 3 FT. TO 4 FT. FROM EACH END) TO PROTECT AGAINST INFILTRATION AND EROSION.

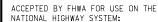
GRANULAR FILL MATERIAL IS NOT PAID FOR DIRECTLY, BUT IS SUBSIDIARY TO THE LINEAR FEET OF CULVERT.

GRANULAR MATERIAL SHALL MEET ASTM D 2487 (SOIL GROUP AS SHOWN IN TABLE 1). MATERIAL SHALL BE COMPACTED TO AT LEAST 90% PROCTOR TEST DENSITY.

PERCENT COMPACTION SHALL BE DETERMINED IN ACCORDANCE WITH NDOR STANDARD TEST METHOD T 99.



STANDARD PLAN NO. 411-R2 BEDDING AND BACKFILL REQUIREMENTS FOR MCCMP PCCMP, & PLASTIC PIPE





TOP OF SUBGRADE

── NATURAL GROUND

- COMPACTED GRANULAR MATERIAL

C/2 ---

LIFT LINES AT 6" MAX. INTERVALS (TYP.)

SEE ROADBED

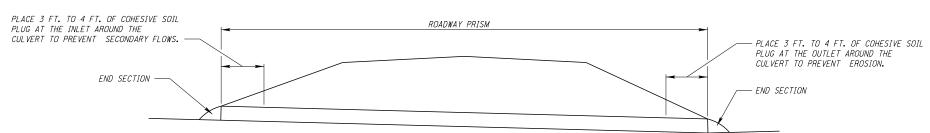
REQUIREMENTS

COMPACTION

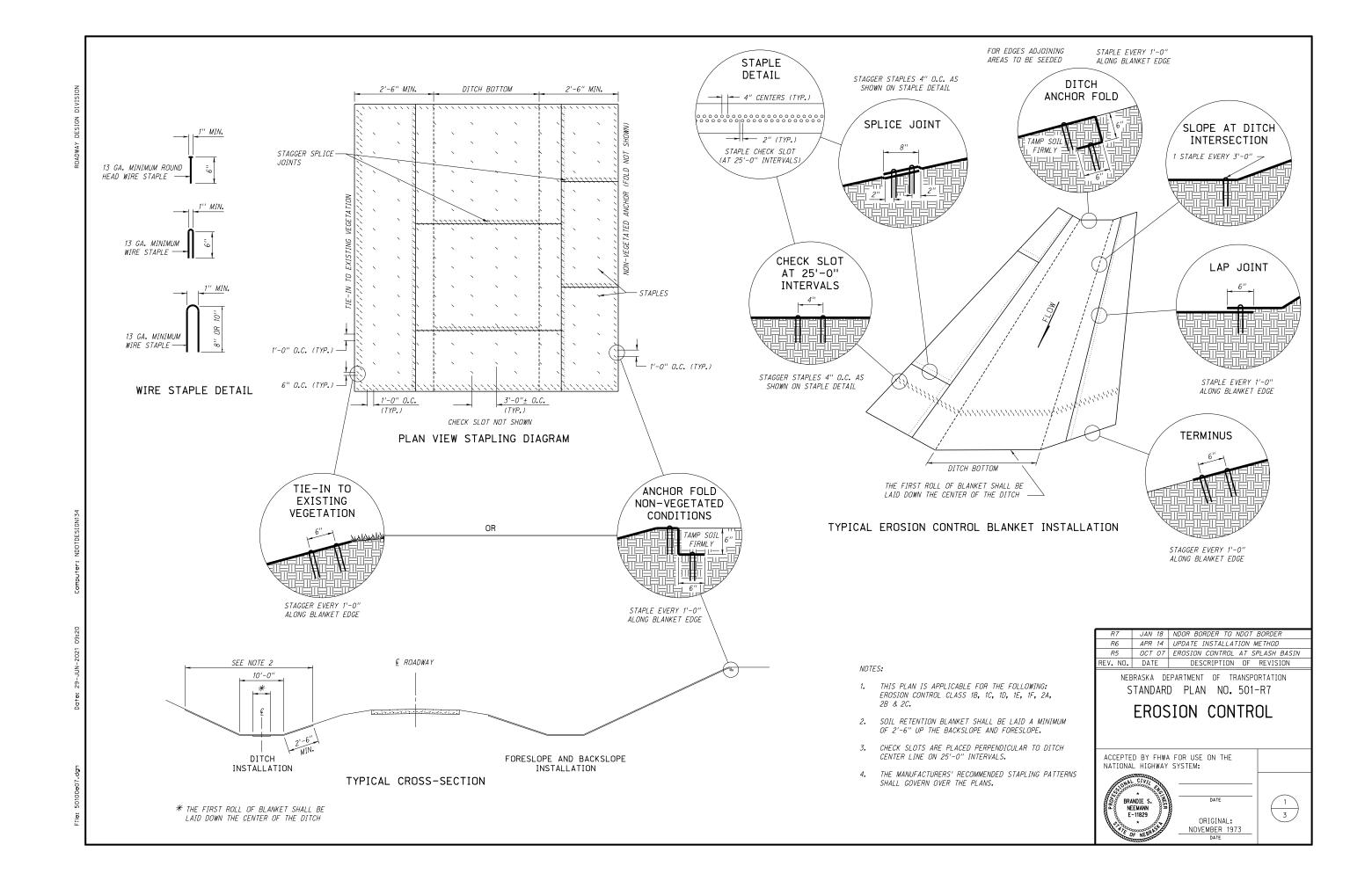


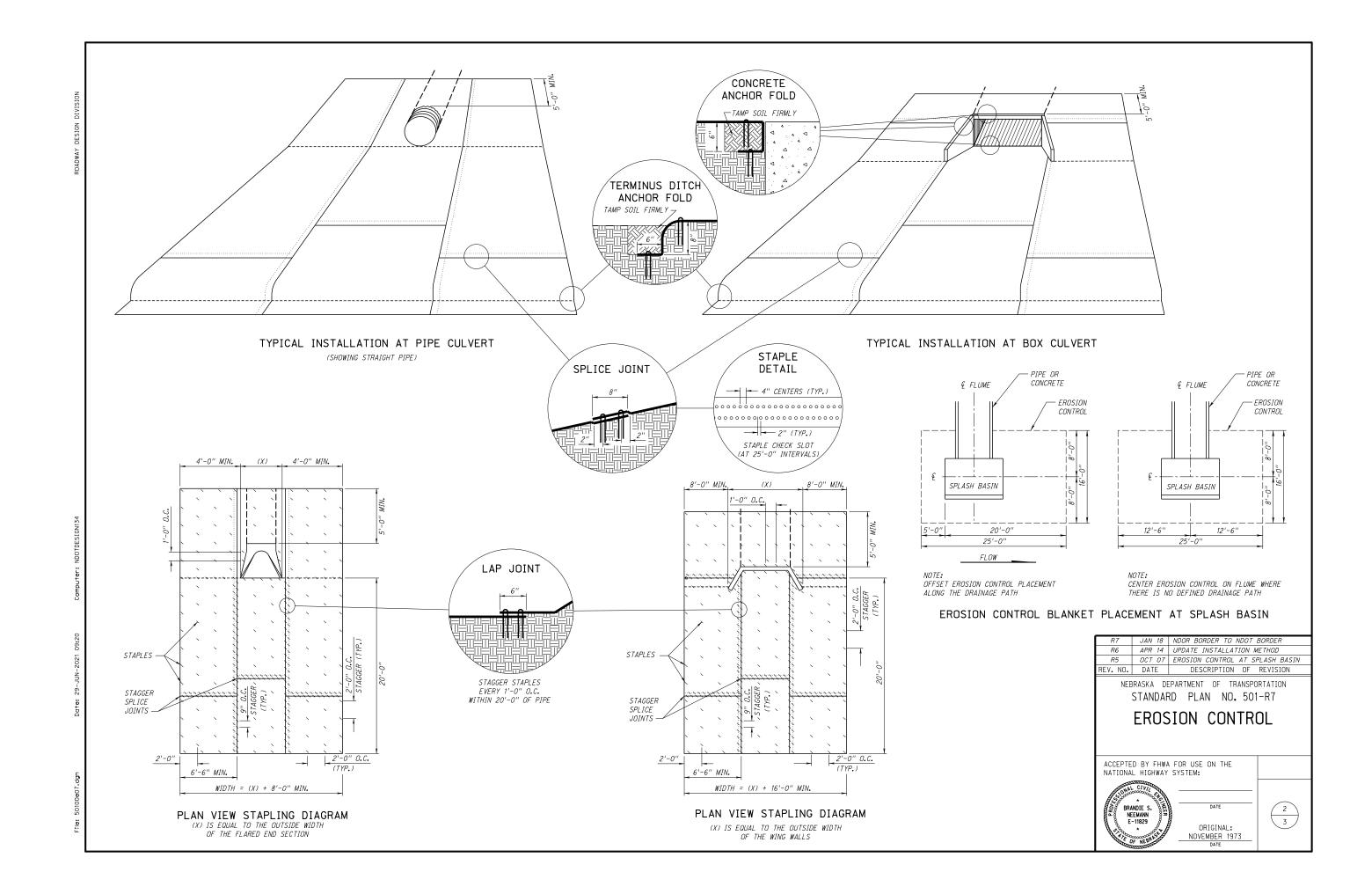
JUNE 6, 2008

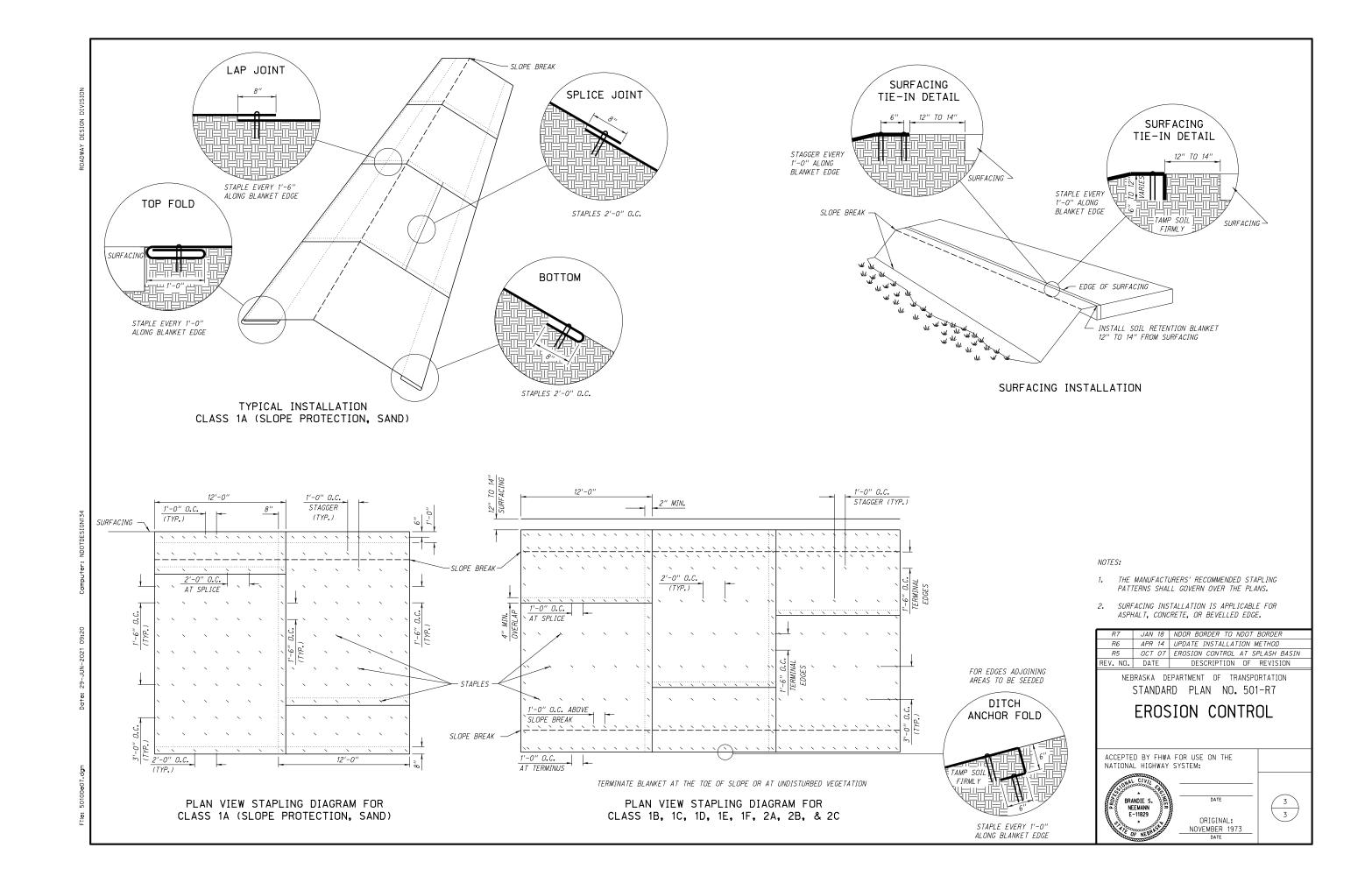


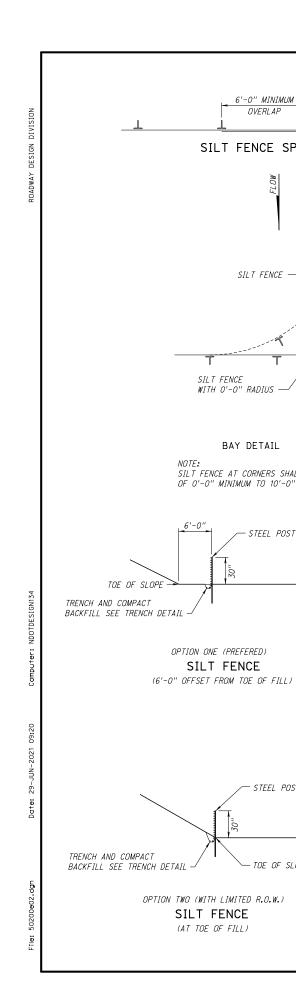


LIMITS OF BEDDING AND BACKFILL









6'-0" MINIMUM OVERLAP

SILT FENCE SPLICE

SILT FENCE

SILT FENCE

OPTION ONE (PREFERED)

SILT FENCE

(AT TOE OF FILL)

SILT FENCE

WITH O'-O" RADIUS

BAY DETAIL

OF O'-O" MINIMUM TO 10'-O" MAXIMUM

SILT FENCE AT CORNERS SHALL HAVE A RADIUS

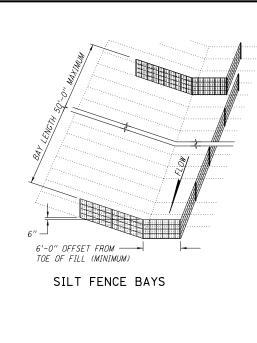
- STEEL POST

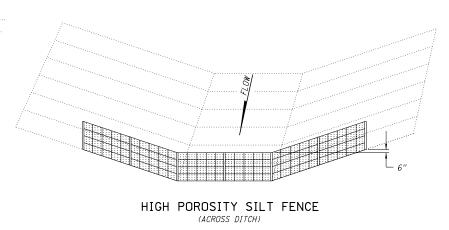
STEEL POST

- TOE OF SLOPE

BAY

BAY





FABRIC \*\*\*\*6" × 1" WIRE STAPLE PLACED AT

SILT FENCE (UNDER BRIDGE)

— SILT FENCE

POST SPACING 6'-0" MAXIMUM MULTIPLE BAYS MAY BE USED

TRENCH DETAIL

\* \* \* SILT FENCE MAY ALSO BE INSTALLED WITH A SILT FENCE PLOW. NO STAPLING IS REQUIRED WHEN THE SILT FENCE PLOW IS USED.

NOTES:

SILT FENCE SHOULD BE 30" ABOVE GRADE (MAY VARY)

SILT FENCE MINIMUM ROLL WIDTH: LOW POROSITY = 42" HIGH POROSITY = 42" LOW PROFILE = 36" COIR SILT FENCE = 36"

STEEL STUDDED "T" LINE POSTS 5'-6" LENGTH; 6'-0" MAXIMUM SPACING.

FOR EACH STEEL STUDDED "T" LINE POST, 3 PLASTIC CABLE TIES ARE REQUIRED.

2" × 2" × 6'-0" NOMINAL WOOD STAKES SPACING, 6'-0" MAXIMUM ON CENTER DRIVEN UNTIL FIRM.

R2	JAN 18	NDOR BORDER TO NDOT BORDER
R1	APR 14	STEEL POST INSTALLATION
REV. NO.	DATE	DESCRIPTION OF REVISION

NEBRASKA DEPARTMENT OF TRANSPORTATION STANDARD PLAN NO. 502-R2

SILT FENCE DETAILS



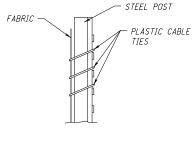


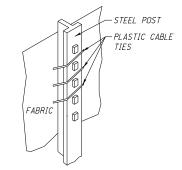
ORIGINAL:

2 / DECEMBER 2006

-STEEL POST SILT FENCE -PLASTIC ZIP TIES COMPACTION ZONE (50 LB TENSILE) LOCATED ON TOP 8" MACHINE SLICE 8" TO 12" DEPTH

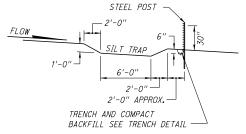
SILT FENCE MACHINE SLICED



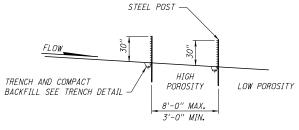


PROFILE VIEW ATTACHMENT TO POST

BACK VIEW ATTACHMENT TO POST

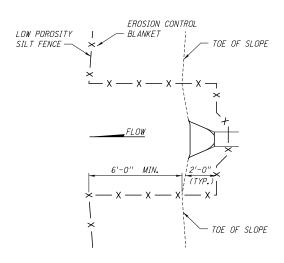


HIGH POROSITY SILT (ACROSS DITCH)



FENCE WITH SILT TRAP

SILT FENCE (ACROSS DITCH)



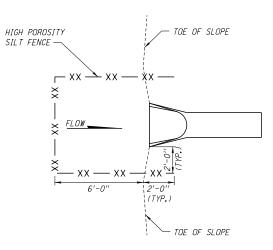
#### SILT FENCE OUTLET PROTECTION

NOTES:

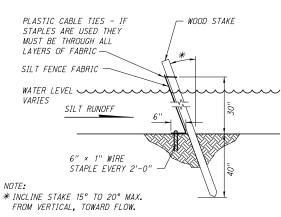
1. SILT FENCE SHOULD BE BROUGHT FLUSH WITH WING WALLS ON BOX CULVERTS IF IT CAN NOT BE INSTALLED ABOVE THE BOX CULVERT.

2. IF APPLICABLE, SILT FENCE AROUND THE CULVERT SHOULD BE ADJUSTED TO ALLOW FOR THE INSTALLATION OF EROSION CONTROL AS SHOWN IN STANDARD PLAN 501.

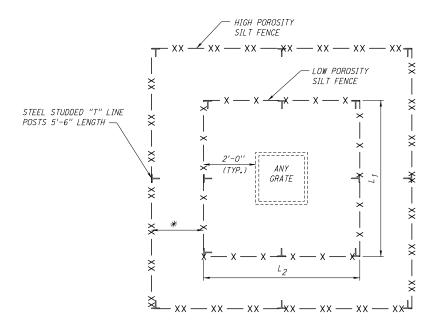
3. SILT CHECKS MAY USED IN PLACE OF SILT FENCE ABOVE THE OPENING OF A CULVERT, AS SHOWN IN SPECIAL PLAN C.



SILT FENCE INLET PROTECTION



SILT FENCE (WET & BELOW WATER INSTALLATION)

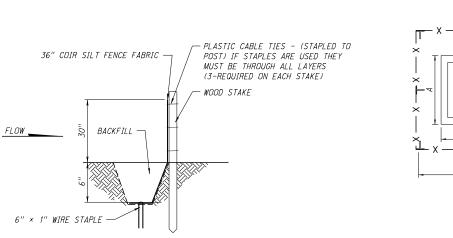


L1 & L2 = OUTSIDE OF WALL + 4'-0"

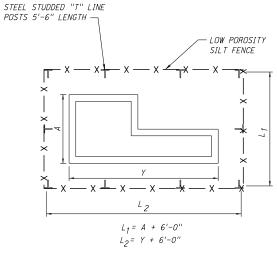
#### PLAN VIEW

#### SILT FENCE FOR GRATE, AREA, MEDIAN INLETS OR JUNCTION BOXES

\* 3'-0" IF POSSIBLE (MAY VARY)



COIR SILT FENCE - ON WOOD POSTS - DRY INSTALLATION



PLAN VIEW SILT FENCE CURB INLET

R2	JAN 18	NDOR BORDER TO NDOT BORDER
R1	APR 14	STEEL POST INSTALLATION
REV. NO.	DATE	DESCRIPTION OF REVISION

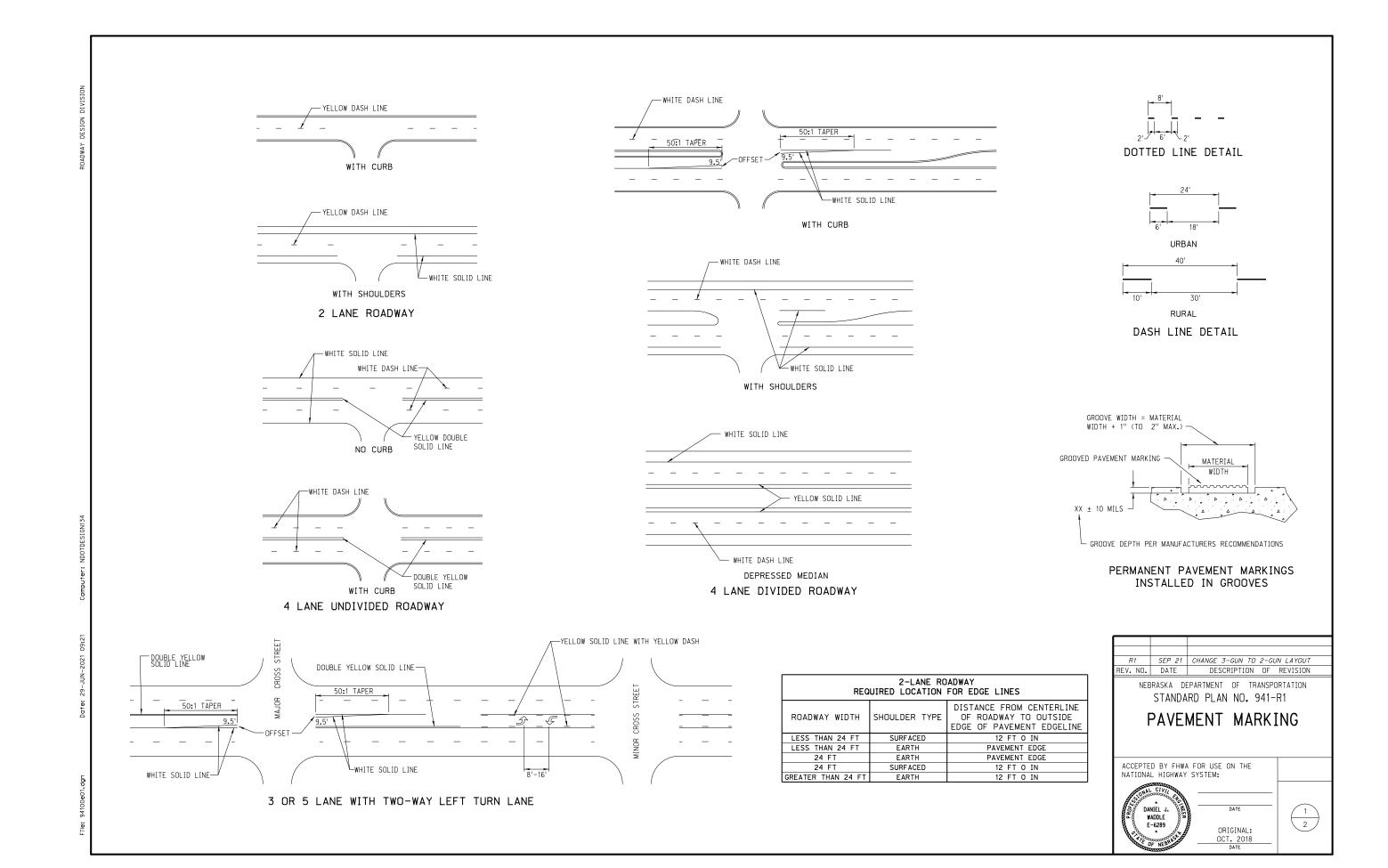
NEBRASKA DEPARTMENT OF TRANSPORTATION STANDARD PLAN NO. 502-R2

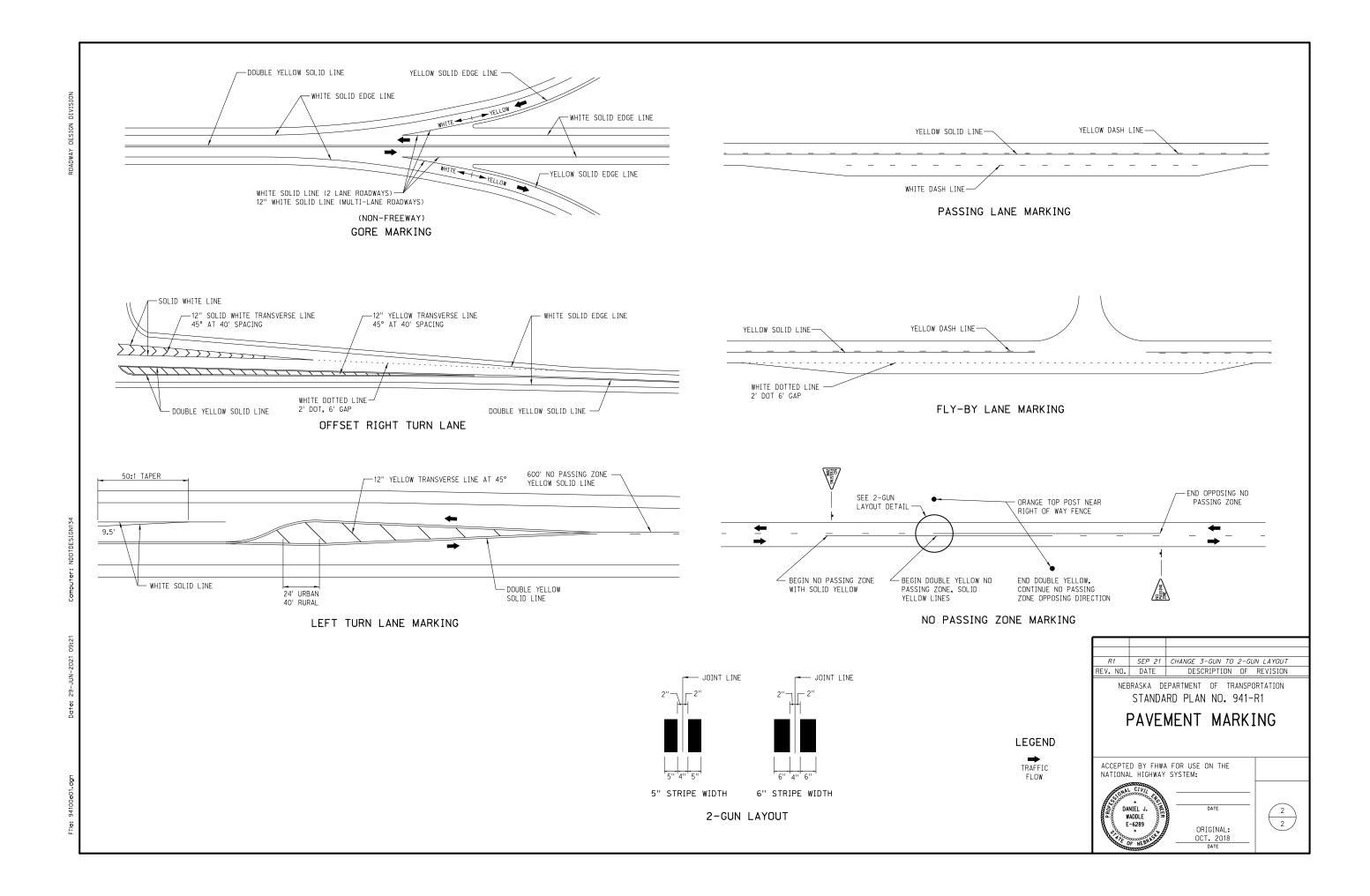
## SILT FENCE DETAILS

2 /

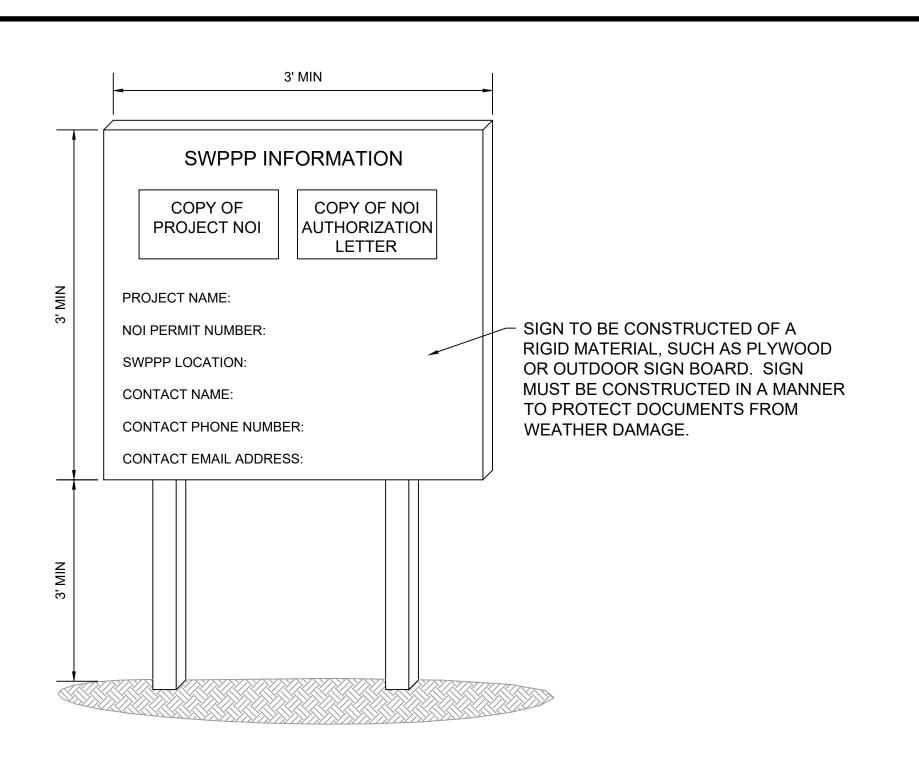




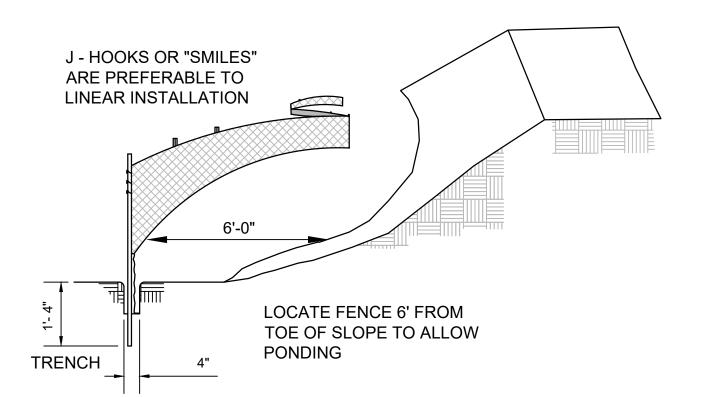


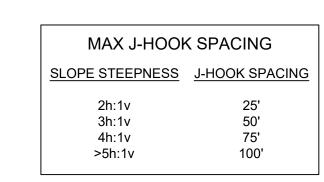


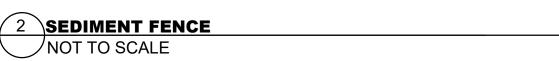




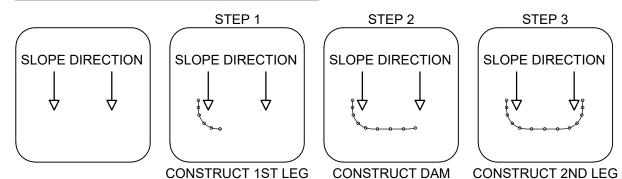
TYPICAL SIGN BOARD DETAIL NOT TO SCALE



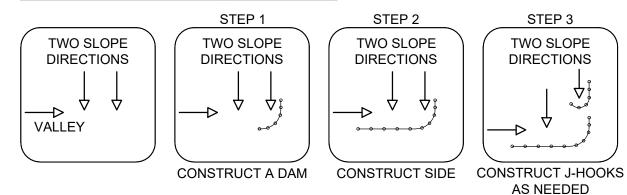




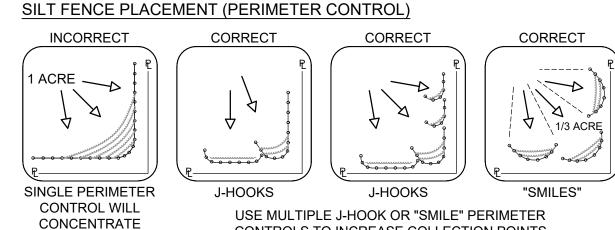
# SILT FENCE PLACEMENT (ONE SLOPE)



## SILT FENCE PLACEMENT (TWO SLOPES)



SEDIMENT RUNOFF

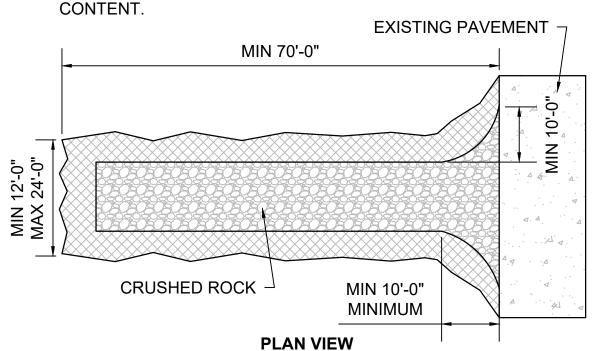


CONTROLS TO INCREASE COLLECTION POINTS

PLACE CLEAN CRUSHED ROCK (2" TO 3.5") OVER GEOTEXTILE FILTER FABRIC. DO NOT USE RECYCLED

3 CONSTRUCTION ENTRANCE

✓ NOT TO SCALE



MIN 12'-0" MAX 24'-0" CRUSHED ROCK EXISTING GROUND -**SECTION** MIN 70'-0" 3'-0" CRUSHED ROCK <sup>\(\triangle\)</sup> **EXISTING GROUND EXISTING PAVEMENT** 

## **SEDIMENT AND EROSION CONTROL NOTES:**

**SIDE VIEW** 

## CONSTRUCTION SEQUENCING:

THE CONTRACTOR IS RESPONSIBLE FOR FOLLOWING THE CONDITIONS OF THE NPDES PERMIT AND IMPLEMENTING THE STORMWATER POLLUTION PREVENTION PLAN. ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE INSTALLED IN ACCORDANCE WITH DIVISION 800 OF THE NEBRASKA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION. THE PERMIT NUMBER, CONTACT PHONE NUMBER, AND SWPPP SHALL BE POSTED ON A SIGN BOARD AT THE CONSTRUCTION ENTRANCE. RECORDS SHALL BE MAINTAINED FOR DATES WHEN MAJOR GRADING ACTIVITIES OCCUR, CONSTRUCTION ACTIVITIES TEMPORARILY OR PERMANENTLY CEASE ON A PORTION OF THE SITE AND STABILIZATION MEASURES ARE INITIATED. CONSTRUCTION SEQUENCING SHALL COMMENCE AS FOLLOWS:

- 1. CONSTRUCTION ENTRANCES (DETAIL 2), OR APPROVED ALTERNATIVE, SHALL BE INSTALLED AT THE LOCATIONS SHOWN ON THE PLANS. MATERIAL SHALL BE 2" - 3.5" CLEAN ROCK. RECYCLED CONTENT WILL NOT BE ACCEPTED.
- 2. INSTALL SEDIMENT FENCING (DETAIL 3), OR OTHER APPROVED PERIMETER CONTROL MEASURES AS NECESSARY AROUND STAGING (NOT SHOWN) AND SPOIL AREAS.
- 3. GRADE THE PROJECT AND CONSTRUCT STRUCTURES AS SHOWN ON THE PLANS.
- 4. PERFORM TEMPORARY SEEDING AND MULCHING IN ANY INACTIVE DISTURBED AREAS FOR PERIODS EXCEEDING FOURTEEN (14) CALENDAR DAYS.
- 5. REMOVE ANY REMAINING WASTE MATERIALS FROM CLEARING AND GRUBBING
- 6. REMOVE SEDIMENT FENCE AND ANY REMAINING TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES.
- 7. PERFORM SEEDING AND PROTECT WITH HYDROMULCH AT THE COMPLETION OF THE PROJECT.
- 8. RECONSTRUCT THE CONSTRUCTION ENTRANCE FOR FUTURE SITE ACCESS.

# MAINTENANCE:

- 1. UNLESS OTHERWISE INDICATED, ALL EROSION AND SEDIMENT CONTROL PRACTICES AND STORMWATER MANAGEMENT PRACTICES SHALL BE CONSTRUCTED AND MAINTAINED IN ACCORDANCE WITH DIVISION 800 OF THE NEBRASKA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION.
- 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF THE EROSION AND SEDIMENT CONTROL MEASURES UNTIL FINAL SITE STABILIZATION IS ACHIEVED.
- 3. ALL SEDIMENT AND EROSION CONTROL PRACTICES WILL BE INSPECTED AT LEAST ONCE EVERY SEVEN (7) CALENDAR DAYS. OR EVERY FOURTEEN (14) CALENDAR DAYS AND AFTER ANY STORM EVENT OF GREATER THAN 0.25 INCHES OF PRECIPITATION DURING ANY 24-HOUR PERIOD BY QUALIFIED PERSONNEL ESTABLISHED BY THE CONTRACTOR . ANY NECESSARY REPAIRS, SEDIMENT REMOVAL OR CLEANUP TO MAINTAIN THE EFFECTIVENESS OF THE BEST MANAGEMENT PRACTICES SHALL BE MADE BEFORE THE NEXT STORM EVENT AND/OR WITHIN SEVEN (7) CALENDAR DAYS FROM THE INSPECTION DATE. SEDIMENT FROM SEDIMENT TRAPS OR SEDIMENTATION PONDS MUST BE REMOVED WHEN DESIGN CAPACITY HAS BEEN REDUCED BY 50 PERCENT. INSPECTION REPORTS SHALL BE GENERATED AND DELAYS SHALL BE DOCUMENTED WITH SUFFICIENT DETAIL AS TO EXPLAIN THE REASON FOR THE DELAY.
- 4. CONTRACTOR SHALL LIMIT THE TRACKING OF MUD AND DEBRIS ONTO NEIGHBORING ROADS. ANY ACCUMULATION OF MUD OR DEBRIS AS A RESULT OF THE CONSTRUCTION ON SHALL BE REMOVED ON A DAILY BASIS AND IMMEDIATELY AFTER SIGNIFICANT BUILDUP.
- 5. FOLLOWING SOIL DISTURBANCE, PERMANENT OR TEMPORARY STABILIZATION SHALL BE COMPLETED AT CONTRACTOR'S EXPENSE WITHIN FOURTEEN (14) CALENDAR DAYS TO THE SURFACE OF ALL PERIMETER SEDIMENT CONTROLS, TOPSOIL STOCKPILES, AND ANY OTHER DISTURBED OR GRADED AREAS ON THE PROJECT SITED WHICH ARE NOT BEING USED FOR MATERIAL STORAGE, OR ON WHICH ACTUAL EARTH MOVING ACTIVITIES ARE NOT BEING PERFORMED.
- 6. FINAL STABILIZATION OCCURS ONE HUNDRED EIGHTY (180) CALENDAR DAYS AFTER CONSTRUCTION ACTIVITY HAS CEASED, VEGETATIVE COVER HAS A MINIMUM 70% DENSITY, TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES HAVE BEEN REMOVED, AND PROPERTIES HAVE BEEN RETURNED TO THE PREEXISTING STATE.

9/1/2023° RICHARD J. KRUSHENISKY

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EROSION



The FLATWATER GROUP Inc. 8200 Cody Drive, Suite A Lincoln, NE 68512 402-435-5441 CA-1145

1-Sep-23 ESIGNED BY RJK RAWN BY STAFF HECKED BY RJK PROJECT NUMBER

EC-2

AS SHOWN

SCALE

SHEET NUMBER